

# MECHANICS OF SOLIDS

Solved Problems

Solved by;

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## AISC SPECIFICATION

### FOR STEEL

$$C_c = \sqrt{\frac{2\pi^2 E}{F_y}}$$

If  $C_c < \frac{L_e}{r}$  then  $S_{ux} = \frac{12\pi^2 E}{23 \left(\frac{L_e}{r}\right)^2}$

If  $C_c > \frac{L_e}{r}$  then

$$S_{ux} = \left(1 - \frac{C_c^2}{\left(\frac{L_e}{r}\right)^2}\right) \frac{F_y}{F_s}$$

$$F.S. = \frac{5}{3} + \frac{3 \left(\frac{L_e}{r}\right)^2}{8 C_c^2} - \frac{\left(\frac{L_e}{r}\right)^4}{8 C_c^4}$$

### FOR ALUMINIUM

US Customary

$$S_{ux} = 30.7 \text{ ksi}$$

$$\frac{L_e}{r} < 12$$

SI

$$S_{ux} = 193.0 \text{ MPa}$$

$$S_{ux} = (30.7 - 0.23 \frac{L_e}{r}) \text{ ksi} \quad \frac{L_e}{r} < 12 \quad S_{ux} = (212 - 1.59 \frac{L_e}{r}) \text{ MPa}$$

$$S_{ux} = \frac{54,000 \text{ ksi}}{\left(\frac{L_e}{r}\right)^2}$$

$$\frac{L_e}{r} > 12$$

$$S_{ux} = \frac{372 \times 10^3 \text{ MPa}}{\left(\frac{L_e}{r}\right)^2}$$

### FOR WOOD

$$S_{ux} = \frac{5.49 E}{\left(\frac{L_e}{r}\right)^2}$$

$$S_{ux} = \frac{0.3 E}{\left(\frac{L_e}{r}\right)^2}$$

$$r = \sqrt{\frac{I}{A}} = \sqrt{\frac{\pi d^4}{4 \pi d^2}} = \frac{d}{2}$$

Masood  
Akhtar

CHAPT IICOLOUMNS

$$M = Py \quad (1)$$

$$y = A \cos kx + B \sin kx \quad (2)$$

$$y = e \cos kx + e \tan \frac{kL}{2} \sin kx$$

$$y_{\max} = e \sec \frac{kL}{2}$$

$$S = \frac{P}{A} \left( 1 + \frac{ec}{R^2} \left( \sec \frac{L}{R} \sqrt{\frac{P}{4AE}} \right) \right)$$

MASOOD AKHTAR  
SKANZ

$$M = Py \Rightarrow y = A \cos kx + B \sin kx$$

$$y = e \cos kx + e \tan \frac{kL}{2} \sin kx$$

$$y_{\max} = e \sec \frac{kL}{2}$$

$$S = \frac{P}{A} \left( 1 + \frac{ec}{R^2} \left( \sec \frac{L}{R} \sqrt{\frac{P}{4AE}} \right) \right)$$

$$\sin kL = 0$$

$$kL = n\pi$$

$$\sqrt{\frac{P}{EI}} = \frac{n\pi}{L} \Rightarrow \frac{P}{EI} = \frac{n^2 \pi^2}{L^2}$$

$$P = \frac{n^2 \pi^2 EI}{L^2}$$

Critical

$$P_{cr} = \frac{\pi^2 EI}{L^2}$$

Slenderness Ratio  $\frac{L_0}{R} = \sqrt{\frac{\pi^2 EI}{S_y P}}$

(2)

GIVEN

PB 34  $D = 1.5m$ ,  $A = 300mm^2 = 300 \times 10^{-6} m^2$

$S_{tmax} = 120 \times 10^6 Pa$ ,  $n = 30m$ ,  $\rho = 1000 kg/m^3$

REQUIRED spacing =  $L = ?$

SOL

Since  $P = A \rho h = 1000 \times 9.81 \times 30$

$= 294300 N$

$F = 2P = PDL$  (small P)

$P = \frac{PDL}{2}$

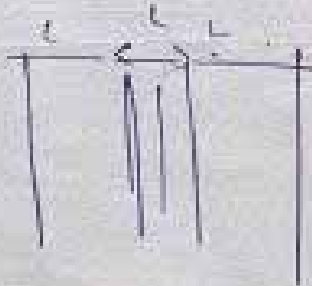
So  $S_{tmax} = \frac{P}{A} = \frac{PDL}{2A}$

So  $L = \frac{S_{tmax} \times 2 \times A}{P \times D}$

$= \frac{120 \times 10^6 \times 2 \times 300 \times 10^{-6}}{294300 \times 1.5}$

$\underline{L} = \underline{70000} = 0.176m$

so spacing = 0.176m



The diagram illustrates a series of vertical bars. The first bar is on the left, followed by a gap of length L, then a second bar, followed by another gap of length L, and finally a third bar. This represents the spacing between reinforcement bars in a concrete slab.

(3)

PD, 133      Given       $D = 400 \text{ mm} = 0.4 \text{ m}$   
 $t = 20 \text{ mm} = 0.02 \text{ m}$   
 $P = 4.5 \times 10^6 \text{ Pa}$

REQ      ①  $S_t$  &  $S_u$   
 ②  $P$  if  $S_t$  or  $S_u = 120 \times 10^6 \text{ Pa}$

Sol

① Since  
 tangential stress  $= S_t = \frac{PD}{2t}$

$$S_t = \frac{4.5 \times 10^6 \times 0.4}{2 \times 0.02} = 45 \text{ MPa}$$

② Since       $S_u = \frac{1}{2} S_t$        $\left( S_u = \frac{PD}{4t} = \frac{PD}{2t} \times \frac{1}{2} \right)$

$$= \frac{1}{2} \times 45 = 22.5 \text{ MPa}$$

③ Let       $S_t = 120 \times 10^6 \text{ Pa}$       so

$$S_t = \frac{PD}{2t} \Rightarrow P = \frac{S_t \times 2t}{D} = \frac{120 \times 10^6 \times 0.02}{0.4}$$

$$P = 12 \text{ MPa}$$

④ For       $S_u = 120 \times 10^6 \text{ Pa}$

$$S_u = \frac{PD}{4t}$$

$$P = \frac{S_u \times 4t}{D} = \frac{120 \times 10^6 \times 4 \times 0.02}{0.4}$$

$$P = 24 \text{ MPa}$$

So       $P = 12 \text{ MPa}$  is safe

PD 134

GIVEN

$$D = 4 \text{ ft} = 4 \times 12 = 48 \text{ in}$$

$$t = \frac{5}{16} \text{ in}$$

$$S_{\text{max}} = 9000 \text{ psi}$$

REQ  $P = ?$

SOL Since for spherical tanks

$$S_{\text{max}} = \frac{PD}{4t}$$

So 
$$P = \frac{S_{\text{max}} \times 4t}{D} = \frac{9000 \times 4 \times \frac{5}{16}}{48}$$

$$P = 234.3 \text{ psi}$$


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PD 136

GIVEN

$$t = 20 \text{ mm} = 0.02 \text{ m}, D = 0.45 \text{ m}$$

$$L = 2 \text{ m}, S_L = 140 \text{ MPa}, S_t = 60 \text{ MPa}$$

REQ  $P = ?$

SOL For tangential stress

$$S_t = \frac{PD}{S_t}$$

$$P = \frac{S_t \times 4t}{D} = \frac{60 \times 10^6 \times 4 \times 0.02}{0.45}$$

$$P = 53 \text{ MPa}$$

For long. stress

$$S_L = \frac{PD}{4t} \Rightarrow P = \frac{S_L \times 4t}{D}$$

$$P = \frac{140 \times 10^6 \times 4 \times 0.02}{0.45} = 240 \text{ MPa}$$

So  $P = 53 \text{ MPa} \rightarrow \text{Answer}$

Pb 137      Given       $D = 22.5 \text{ ft} = 264 \text{ in}$ ,  $t = \frac{1}{2} \text{ in}$   
 $S_e = 6000 \text{ psi}$ ,  $\gamma_w = 62.4 \text{ lb/ft}^3$

Req       $h = ?$        $= \frac{62.4}{123} = 0.036$

Sol      since  $P = \rho g h = \gamma_w h$   
 $\& S_e = \frac{PD}{2t} = \frac{\gamma_w h D}{2t}$   
 $h = \frac{S_e \times 2t}{\gamma_w D} = \frac{6000 \times 2 \times 0.5}{62.4 \times 264}$   
 $h = 631.31 \text{ in} = 52.6 \text{ ft}$

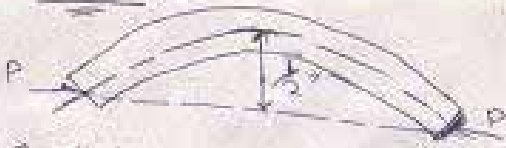
Pb 138

$S_u = 33 \text{ kips/ft}^2$ ,  $P = 150 \text{ psi}$   
 $= \frac{33 \times 10^3}{144}$   
 $0.289 \times 10^3 \text{ psi}$


$S_u = \frac{PD}{4t}$       \*

41072007)  
PD 902

Case a




Case b



REQD  $\frac{(S_{max})_{bent}}{(S_{max})_{straight}} = \frac{P}{P}$

Sol (Case a)

Calculate section properties



$$I = \frac{bh^3}{12} = \frac{\frac{1}{2} \times \frac{1}{2}^3}{12} = \frac{0.54 \text{ in}^4}{12}$$

$$C = h/2 = \frac{0.5}{2} = 0.25 \text{ in}$$

$$A = 0.5 \times 0.5 = 0.25 \text{ in}^2$$

$$M = 0.5 \times P$$

Axial stress is  $S_A = P/A = P/0.25 = -4P$

Flexure stress is  $S_f = \frac{Mc}{I} = \frac{0.5P \times 0.25}{\frac{0.54}{12}} = 24P$

Point A is under tension due to flexure

$$\therefore S_A = -S_A + S_f$$

$$= -4P + 24P = 20P$$

AND  $S_B = -4P - 24P = -28P$

Max stress is  $S = -28P$

CASE B Straight bar

In this case the bar is subjected to axial stress only and not flexure

$$\therefore S_{max} = -4P$$



$$\text{Now } \frac{(\sigma_{\max})_{\text{bent}}}{(\sigma_{\max})_{\text{straight}}} = \frac{20P}{14P} = 7:1$$

This shows that max stress in bent bar is 7 times than that of straight.

Pb 903  
 20mm  
 40mm  
 150mm  
 500mm

$(\sigma_{ax})_{\text{tension}} = 40 \text{ MPa}$   
 $(\sigma_{ax})_{\text{comp}} = 80 \text{ MPa}$   
 allowable  
Reqd  $P = ?$   
 safe

Solution Calculate section properties

$I = \frac{bh^3}{12} = \frac{0.04 \times 0.2^3}{12} = 2.67 \times 10^{-6} \text{ m}^4$

$y = 150/2 = 75 \text{ mm} = 0.075 \text{ m}$

$A = bh = 0.04 \times 0.2 = 0.008 \text{ m}^2$

$M = 0.05 \times P = 0.05P$

$c = \frac{20}{2} = 0.01 \text{ m}$

Now Axial stress  $\sigma$

$\sigma_a = -P/A = -P/0.008 = -125P$

Flexure stress  $\sigma = \frac{My}{I} = \frac{0.05P \times 0.1}{2.67 \times 10^{-6}} = 187.27P$

PLA is under compression due to flexure stress


$\sigma_A = -\sigma_a + \sigma_f = -125P + 187.27P = 62.27P$

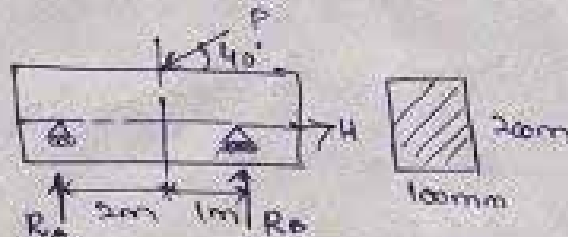
$\sigma_B = -\sigma_a - \sigma_f = -125P - 187.27P = -312.27P$

$$\begin{aligned}
 \sigma_A &= (\sigma_{all})_{comp} = 80 \text{ MPa} = 312.27 \text{ P} \\
 \therefore P &= 256.2 \text{ kN} \\
 \sigma_B &= (\sigma_{all})_{tension} = 40 \text{ MPa} = 62.27 \text{ P} \\
 P &= 642.36 \text{ kN}
 \end{aligned}$$

Safe value of P is least that is 256.2 kN

$\pm P/A = M/I$



PD 905

$\sigma_{max} = 10 \text{ MPa}$  (for both tension and compression)

REQD & First of all resolve P into components

$$P_x = P \cos 40^\circ = 0.766P$$

$$P_y = P \sin 40^\circ = 0.64P$$

Calculate reactions

$$\sum F_y = 0 \uparrow +ve \quad R_A + R_B - P_y = R_A + R_B = 0.64P \quad (1)$$

$$\sum F_x = 0 \rightarrow +ve \quad H - P_x = 0 \quad H = P_x \Rightarrow H = 0.766P$$

$$\sum M_A = 0 \quad \curvearrowright +ve \quad -R_B \times 3 + R_A \times 2 - P_x \times 0.1 = 0$$

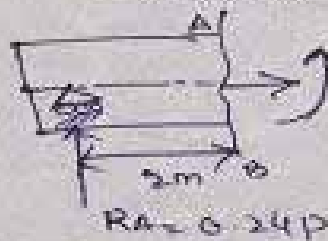
$$R_B = \frac{0.64P \times 2 - 0.766P \times 0.1}{3} = 0.40P$$

$$R_A = 0.64P - 0.40P = 0.24P$$

Calculate section properties

$$I = \frac{bh^3}{12} = \frac{0.1 \times 0.2^3}{12} = 6.67 \times 10^{-5} \text{ m}^4$$

$$A = 0.1 \times 0.2 = 0.02 \text{ m}^2 \quad \left\{ \begin{array}{l} C = h/2 = 0.1 \text{ m} \\ P_x = 0.766P \\ 0.24P \times 2 = 0.48P \end{array} \right.$$



Axial stress is Tensile i.e

$$S_a = P/A = \frac{P}{\pi d^2/4} = \frac{4P}{\pi d^2} = \frac{0.766P}{0.02} = 38.3P$$

Flexure stress is  $S_f = \frac{Mc}{I} = \frac{0.40 \times 6.1}{6.67 \times 10^{-5}} = 719.64P$

Point A is under compression due to moment

$$\therefore \{ S_A = +S_a - S_f = 38.3P - 719.64P = -681.34P \}$$

$$\therefore S_A = +S_a - S_f = 38.3P - 719.64P = -681.34P$$

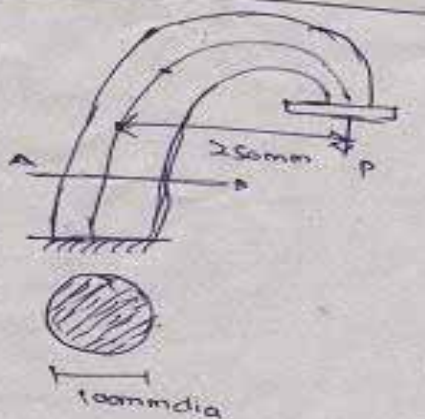
But  $S_A = 10 \text{ MPa}$

$$\text{so } 10 \times 10^6 \text{ Pa} = 681.34P = 681.34P$$

$$P = 13.1 \text{ kN}$$

Least value is safe value of P is  
 $P = 13.1 \text{ kN}$

Pb 907



$$(S_{\text{lim}}) = 10 \text{ MPa}$$

Reqd  $P_{\text{safe}} = ?$

Solution calculate section properties

$$I = \frac{\pi d^4}{64} = \frac{\pi \times 0.1^4}{64} = 4.91 \times 10^{-6} \text{ m}^4$$

$$A = \frac{\pi d^2}{4} = \frac{\pi \times 0.1^2}{4} = 7.86 \times 10^{-3} \text{ m}^2$$

$$e = d/2 = 0.05 \text{ m}$$

Consider equilibrium of section A-B

Axial stress is compressive i.e

$$S_a = -P/A = -\frac{P}{7.76 \times 10^{-3}} = -129.0P$$

$$S_f = \frac{Mc}{I} = \frac{0.25P \times 0.05}{4.91 \times 10^{-6}}$$

$$S_f = 2545.0P$$

At A is under tension due to moment

$$\therefore S_A = -S_a + S_f$$

$$= -129.0P + 2545.0P$$

$$S_A = 2417P \quad \text{put } S_A = 90 \text{ MPa}$$

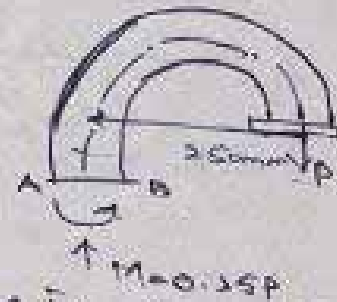
$$90 \times 10^6 \text{ Pa} = 2417P \quad \therefore P = 33 \text{ kN}$$

$$S_B = -129.0P - 2545.0P = -2674.6P$$

$$90 \times 10^6 \text{ Pa} = 2674.6P$$

$$P = 29.9 \text{ kN}$$

safe value of P is 29.9 kN

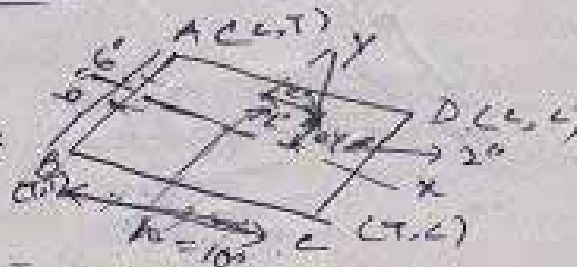


CH#9.  
 ② KERN OF A SECTION

9/9/2009 ①

PD 918

Given  $P = 12k$



RQD  
 Stress at each corner

Sol Since general equation for stress is

$$-\frac{P}{A} \pm \frac{M_x}{I_x} y \pm \frac{M_y}{I_y} x$$

so Dist of all calculating  $M_x, M_y, I_x, I_y$   
 $x, y, A$

$$M_x = P e_y = 12 \times 2 = 24 \text{ kip-in} = 24 \times 10^3 \text{ lb-in}$$

$$M_y = P e_x = 12 \times 10^3 \times 1 = 12 \times 10^3 \text{ lb-in}$$

$$I_x = \frac{b h^3}{12} = \frac{10 \times 6^3}{12} = 180 \text{ in}^4$$

$$I_y = \frac{b^3 h}{12} = \frac{6 \times 10^3}{12} = 500 \text{ in}^4$$

$$A = 6 \times 10 = 60 \text{ in}^2 \quad x = \frac{h}{2} = \frac{10}{2} = 5 \quad y = \frac{b}{2} = 3$$

so calculating stress at A, B, C, D.

$$\sigma_A = \frac{P}{A}$$

$$\frac{M_x}{I_x} y = 4000 \text{ psi}$$

$$\frac{M_y}{I_y} x = \frac{12 \times 10^3 \times 5}{500} = 120 \text{ psi}$$

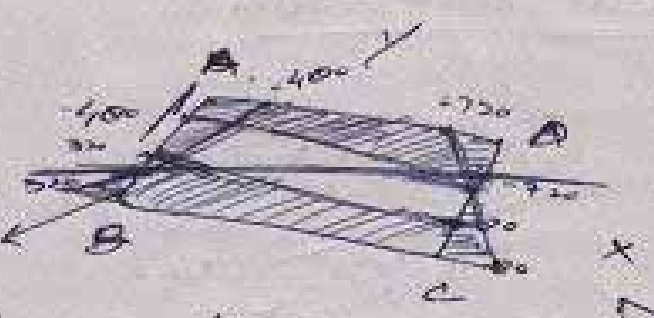
$$P/A = 12 \times 10^3 / 60 = 200 \text{ psi}$$

$$\sigma_A = -\frac{P}{A} - \frac{M_x}{I_x} y + \frac{M_y}{I_y} x$$

$$= -200 - 400 + 120 = -480 \text{ PSI}$$

$$\sigma_B = -P/A + \frac{M_x y}{I_x} + \frac{M_y x}{I_y} = -200 + 400 + 120 = 320 \text{ PSI}$$

$$\sigma_C = -P/A + \frac{M_x y}{I_x} - \frac{M_y x}{I_y} = -200 + 400 - 120 = 80 \text{ PSI}$$

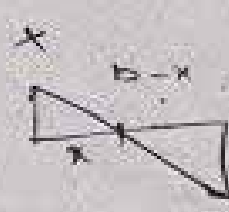
$$\sigma_D = -P/A - \frac{M_x y}{I_x} - \frac{M_y x}{I_y} = -200 - 400 - 120 = -720 \text{ PSI}$$


$$\frac{320}{x} = \frac{480}{6-x}$$

$$320(6-x) = 480x$$

$$1920 - 320x = 480x$$

$$1920 = 800x$$

$$x = 2.4"$$


Neutral axis is at 2.4" along y.



Pb 919

since max tensile stress is  
320 psi

so

$$S_B = \frac{P_{add}}{A}$$

$$\text{or } P_{add} = S_B \times A = 320 \times 60 \\ = 19200 \text{ lbs.}$$

Pb 920

P = 100 kN.

RQD

Padd = ?

To find Padd calculate max  $S_t$   
which is at C i.e.

$$S_c = -\frac{P}{A} + \frac{M_x}{I_x} y + \frac{M_y}{I_y} x.$$

Calculating  $M_x, I_x, M_y, I_y, A, x, y$ .

$$M_x = 100 \times 10^3 \times 0.03 = 3000 \text{ N-m}$$

$$M_y = 100 \times 10^3 \times 0.075 = 7000 \text{ N-m}$$

$$I_x = \frac{0.15 \times 0.3^3}{12} = 0.0004375 \text{ m}^4$$

$$I_y = \frac{0.15^3 \times 0.3}{12} = 0.0003375 \text{ m}^4$$

$$\left\{ \begin{array}{l} x = 0.15 \text{ m} \\ y = 0.075 \text{ m} \\ A = 0.15 \times 0.3 \\ = 0.045 \text{ m}^2 \end{array} \right.$$

$\sigma_c =$   
 so  $P/A = 100 \times 10^3 / 0.045 = 2222222.22$   
 $\frac{M_x}{I_x} y = \frac{3000 \times 0.15}{0.000004375} \times 0.075 = 2666666.667$   
 $\frac{M_y}{I_y} x = \frac{7000 \times 0.075}{0.0003375} = 1533333.333$   
 so  $\sigma_c = -P/A + \frac{M_x}{I_x} y + \frac{M_y}{I_y} x$   
 $= -2222222.22 + 2666666.67 + 1533333.33$   
 $\sigma_c = 3555555.555$   
 so  $\sigma_c = \frac{P_{add}}{A}$   
 so  $P_{add} = 1600000 \text{ N}$

PD 921

$A = 15500 \text{ mm}^2$ ,  $t_w = 13 \text{ mm}$ ,  $t_f = 21.7 \text{ mm}$   
 $b_f = 257 \text{ mm}$ ,  $S_x = 2010 \times 10^3 \text{ mm}^3$ ,  $S_y = 470 \times 10^3 \text{ mm}^3$

From figure the max  $\sigma$  tensile stress is at B so

$$\sigma_B = -\frac{P}{A} + \frac{M_x}{S_x} + \frac{M_y}{S_y}$$

Putting  $\sigma_B = 0$  to calc (kern)

$$0 = -\frac{P}{15500} + \frac{P \bar{y}}{S_x} + \frac{P \bar{x}}{S_y}$$

$$\frac{0}{P} = \frac{P}{P} \left( -\frac{1}{15500} + \frac{\bar{y}}{S_x} + \frac{\bar{x}}{S_y} \right)$$

$$0 = -\frac{1}{15500} + \frac{\bar{y}}{S_x} + \frac{\bar{x}}{S_y}$$

Or  $\frac{\bar{y}}{2010 \times 10^3} + \frac{\bar{x}}{470 \times 10^3} = \frac{1}{15500}$

$$\frac{\bar{y}}{2010 \times 10^3} \times 15500 + \frac{\bar{x}}{470 \times 10^3} \times 15500 = 1$$

$$\frac{\bar{y}}{129.7} + \frac{\bar{x}}{30.3} = 1$$

$$\frac{\bar{y}}{123.6} + \frac{\bar{x}}{30.3} = 1$$

Ex 9.23

$\sigma_x = 0 \text{ MPa}$      $\sigma_y = -40 \text{ MPa}$   
 $\sigma_P = \sigma_T = P$      $\theta = 30^\circ$

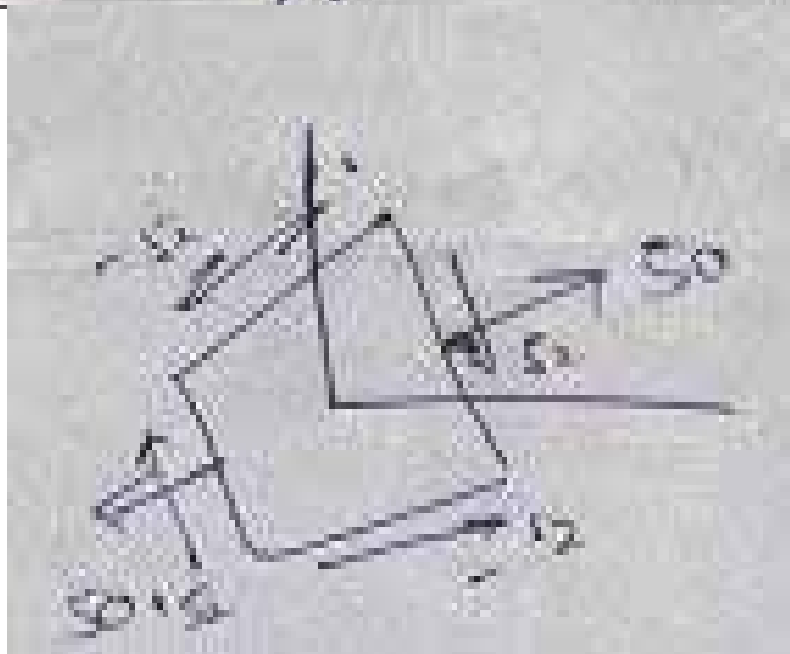
$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

$$= \frac{0 - 40}{2} + \frac{0 - (-40)}{2} \cos 60 - \tau_{xy} \sin 60$$

$$= -20 + 20 \times 0.5 = -20 + 10 = -10 \text{ MPa}$$

4  $\tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$

$$= \frac{0 - (-40)}{2} \sin 60 + \tau_{xy} \cos 60 = 52 \text{ MPa}$$



Pb 924

- Determining normal & shearing stress in
- (a) principal planes (b) The plane of max. in plane shearing stress  $\tau$
- (c) The planes whose normal are at  $36.8^\circ$  &  $126.8^\circ$  with x-axis

(a) Principal planes.

$$\left. \begin{matrix} \sigma_1 \\ \sigma_2 \end{matrix} \right\} \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}$$

$$= \frac{32 + (-10)}{2} \pm \sqrt{\left(\frac{32 + 10}{2}\right)^2 + (-20)^2}$$

$$= 11 \pm \sqrt{2^2 + 400} = 11 \pm \sqrt{841}$$

$$\left. \begin{matrix} \sigma_1 \\ \sigma_2 \end{matrix} \right\} = 11 \pm 29 =$$

$$\sigma_1 = 11 + 29 = 40 \text{ MPa} \quad \& \quad \sigma_2 = 11 - 29 = -18 \text{ MPa}$$

$$\tan 2\theta = \frac{-2\tau_{xy}}{\sigma_x - \sigma_y} = \frac{-2(-20)}{32 + 10} = \frac{40}{42}$$

$$2\theta = \tan^{-1} \frac{40}{42} =$$

$$2\theta = 43.6^\circ \Rightarrow \theta = 21.8^\circ$$

Now shearing stress

$$\tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$= \frac{32 + 10}{2} \sin 43.6 + (-20) \cos 43.6$$

$$= 14.40 - 14.40 = 0$$

⑥ plane of max in plane shearing stress  
is  $T_{xy}$ ,  $T_{yx}$

$$\begin{aligned} T_{xy\max} &= \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (T_{xy})^2} \\ &= \pm \sqrt{\left(\frac{32 - (-10)}{2}\right)^2 + (-20)^2} \\ &= \pm 39 \end{aligned}$$

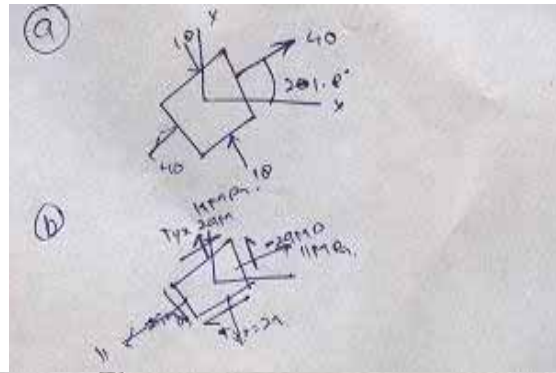
$$\tan 2\theta_s = \frac{\sigma_x - \sigma_y}{2T_{xy}} = \frac{32 + 10}{-40} = -\frac{42}{40}$$

$$\begin{aligned} 2\theta &= -46.3^\circ \\ \theta &= -23.15^\circ \end{aligned}$$

$$\begin{aligned} \sigma &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - T_{xy} \sin 2\theta \\ &= \frac{32 - 10}{2} + \frac{32 + 10}{2} \cos 46.3^\circ - (-20) \sin 46.3^\circ \\ &= 11 + 14.5 - 14.5 = 11 \text{ MPa} \end{aligned}$$

$$\begin{aligned} \sigma &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - T_{xy} \sin 2\theta \\ &= \frac{32 - 10}{2} + \frac{42}{2} \frac{32 + 10}{2} \cos 73.6^\circ - (-20) \sin 73.6^\circ \\ &= 11 + 8.92 + 19.18 = 39.1 \end{aligned}$$

$$\begin{aligned} \tau &= \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + T_{xy} \cos 2\theta \\ &= \frac{32 + 10}{2} \sin 73.6^\circ + (-20) (\cos 73.6^\circ) \\ &= 20.1 - 5.64 = 14.45 \text{ MPa} \end{aligned}$$



pb'  
Q25

(3)

$$\sigma_x = \frac{200 \times 10^3}{0.05 \times 0.1} = \frac{200 \times 10^3}{0.005} = 40 \text{ MPa}$$

$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

$$= \frac{40}{2} + \frac{40}{2} \cos(100) - 0$$

$$= 20 - 3.47 = 16.5 \text{ MPa}$$

$$\tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta = \frac{40}{2} \sin(100) = 19.6 \text{ MPa}$$

PB 926

RQD (a) Plane stresses (max normal stress)  
 (b) stress components at  $30^\circ$

(a)

$$\left. \begin{array}{l} \sigma_1 \\ \sigma_2 \end{array} \right\} \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2}$$

$$= \frac{4000 - 8000}{2} \pm \sqrt{\left(\frac{4000 + 8000}{2}\right)^2 + (6000)^2}$$

$$= -2000 \pm \sqrt{(6000)^2 + (6000)^2} = -2000 \pm \sqrt{72000000}$$

$$= -2000 \pm 8485.28$$

$$\sigma_1 = -2000 + 8485.28 = 6485.28 \text{ psi}$$

$$\sigma_2 = -2000 - 8485.28 = -10485.28 \text{ psi}$$

Now stress components

$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

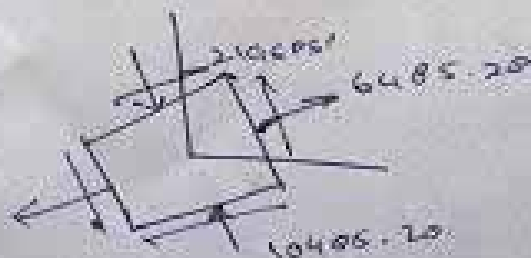
$$= \frac{4000 - 8000}{2} + \frac{4000 + 8000}{2} \cos 60^\circ - (6000 \sin 60^\circ)$$

$$= -2000 + 6000 \times 0.5 + 5196.1 = 6196.1 \text{ psi}$$

$$\tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta$$

$$= 6000 \times 0.66 + (-6000 \times 0.5)$$

$$5196 - 3000 = 2196 \text{ psi}$$





(b) Now finding components.

$$\sin \theta = \frac{c}{r} \Rightarrow \theta = \sin^{-1} \frac{c}{r}$$

$$\sin \theta = \frac{c}{0.40}$$

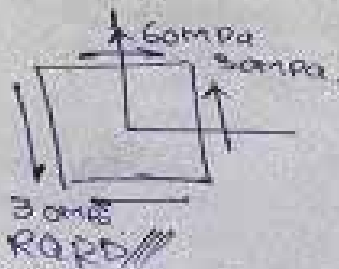
$$\theta = 45^\circ$$

$$\sigma'_x = \sigma_c + R \cos 15^\circ = -2 + 0.19 = -1.81 \text{ ksi}$$

$$\tau_{xy} = R \sin 15^\circ = 0.19 \text{ ksi}$$

←————→

Prob 27



① Principal stress -  $\sigma$  ②  $\tau_{max} = ?$

$$\begin{aligned} \sigma_{1,2} &= \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} \\ &= \frac{60}{2} \pm \sqrt{\left(\frac{-60}{2}\right)^2 + (-30)^2} = 30 \pm \sqrt{(-30)^2 + (-30)^2} \\ &= 30 \pm \sqrt{900 + 900} = 30 \pm 42.426 \end{aligned}$$

$$\sigma_1 = 30 + 42.42 = 72.42 \text{ MPa}$$

$$\sigma_2 = 30 - 42.42 = -12.42 \text{ MPa}$$

Now

$$\begin{aligned} \tau_{max} &= \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} \\ &= \pm 42.42 \text{ MPa} \quad (\text{as above}). \end{aligned}$$

$$\tan 2\theta_n = \frac{-2\tau_{xy}}{\sigma_x - \sigma_y} = \frac{-2(-30)}{-60}$$

$$\tan 2\theta_n = -2$$

$$2\theta = -63.43$$

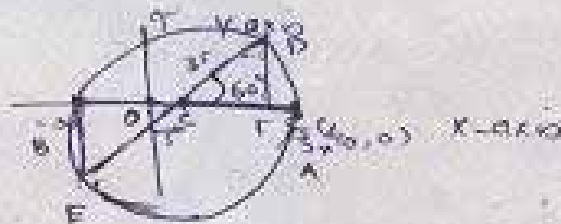
$$\begin{aligned} \tau &= \frac{\sigma_2 - \sigma_1}{2} \sin 2\theta + \tau_{xy} \cos 2\theta \\ &= -30(-\sin 63.43) + (-30 \cos 63.43) \\ &= 26.03 \text{ MPa} \\ \tau &= 40.24 \end{aligned}$$

PBQ 28 Given  $\sigma_x = 40 \text{ MPa}$   $\sigma_y = -30 \text{ MPa}$

Q.0 Stress components on planes whose normal are at  $30^\circ$  &  $120^\circ$  with x-axis

By MOHR'S METHOD

- ①  $C = \frac{\sigma_x + \sigma_y}{2} = \frac{40 - 30}{2} = 5$
- ②  $R = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} = \sqrt{\left(\frac{40 + 30}{2}\right)^2 + 0} = \sqrt{35^2} = 35 \text{ MPa}$
- ③ coordinates are  $(40, 0)$



So stress components at  $60^\circ$  are

$$\begin{aligned} \sigma' &= OC + CA = 5 + 35 = 40 \text{ MPa} \\ &= OC + CF = 5 + 35 \cos 60 = 22.5 \end{aligned}$$

$$\sigma'_{120} = OC - 35 \cos 60 = 5 - 17.5 = -12.5 \text{ MPa}$$

$$\tau_{60} = 35 \sin 60 = 30.3 \text{ MPa}$$

$$\tau_{120} = -30.3 \text{ MPa}$$



pb  
Q20

$$\sigma_x = 40 \text{ MPa} \quad \& \quad \sigma_y = -30 \text{ MPa}$$

compute stress components

$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - 0$$

$$= \frac{40 + (-30)}{2} + \frac{40 - (-30)}{2} \cos 60^\circ = 5 + 35 \times 0.5 = 5 + 17.5$$

$$\tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta = \frac{40 - (-30)}{2} \sin 60^\circ = 35 \times 0.866 = 30.31 \text{ MPa}$$

and at  $480^\circ$

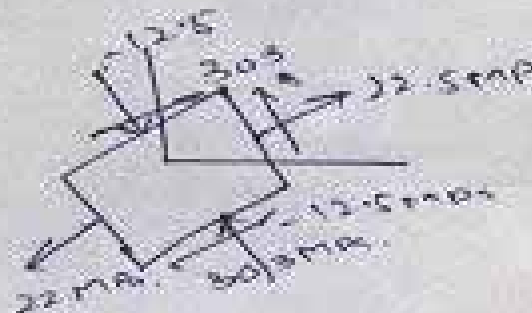
$$\sigma = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta$$

$$= \frac{40 - 30}{2} + \frac{40 + 30}{2} \cos 240^\circ$$

$$\sigma = 5 - 17.5 = -12.5 \text{ MPa}$$

$$\& \quad \tau = \frac{\sigma_x - \sigma_y}{2} \sin 2\theta = \frac{40 + 30}{2} \sin 240^\circ$$

$$= -30.31 \text{ MPa}$$



Prob 9.29 Find stress components at  $30^\circ$  &  $120^\circ$

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} = 0 \pm \sqrt{0 + (-0.000)^2}$$

$$\sigma = 0.000 = 0 \text{ ksi}$$

$$\sigma_1 = \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta$$

$$0 + 0 - (-0.000) \sin 60$$

$$\sigma_1 = + 692.0 \text{ MPa}$$

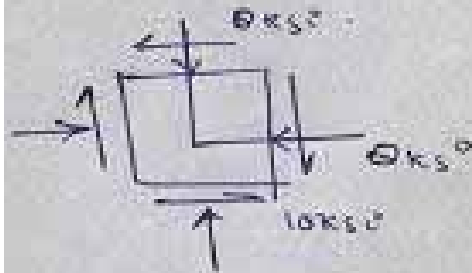
and a)  $\sigma_{120} = -\tau_{xy} \sin 240^\circ$

$$= -(-0.000) \sin 240^\circ$$

$$= - 692.0 \text{ MPa}$$

Pb Q31

QAD normal & shearing stress on the planes whose normal at  $60^\circ$  &  $150^\circ$



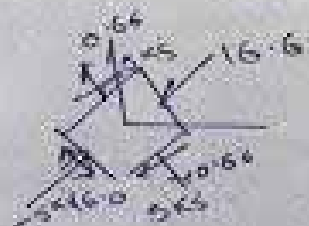
$$\begin{aligned}\sigma_{60} &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta \\ &= \frac{-0.85 - 0.85}{2} + \frac{-0.85 + 0.85}{2} \cos 120^\circ - (10) \sin 120^\circ \\ &= -0.85 + 0 + 0.66 = 0.66 \text{ MPa} = 16.66 \text{ MPa}\end{aligned}$$

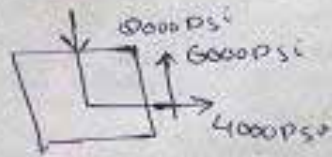
$$\begin{aligned}\sigma_{150} &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta - \tau_{xy} \sin 2\theta \\ &= \frac{-0.85 - 0.85}{2} + \frac{-0.85 + 0.85}{2} \cos 300^\circ - (10) \sin 300^\circ \\ &= -0.85 + 0.66 = 0.66 \text{ MPa}\end{aligned}$$

and

$$\begin{aligned}\tau_{60} &= \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta \\ &= \frac{-0.85 + 0.85}{2} \sin 120^\circ + (10) \cos 120^\circ \\ &= -5 \text{ MPa}\end{aligned}$$

$$\tau_{150} = 0 + (10) \cos 300^\circ = 5 \text{ MPa}$$



Pb Q32Q32 Max in plane shearing stressMOHR METHOD

$$\textcircled{1} \quad C = \frac{\sigma_x + \sigma_y}{2} = \frac{10000 - 6000}{2} = -2000 \text{ psi}$$

$$= -2 \text{ ksi}$$

$$\textcircled{2} \quad R = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} = \sqrt{\left(\frac{10000 + 6000}{2}\right)^2 + (-4000)^2}$$

$$= \sqrt{(8000)^2 + (-4000)^2} = \sqrt{36000000 + 36000000}$$

$$= 0.40520 \cdot 10^4$$

$$= 4.052 \text{ ksi}$$

\textcircled{3} co-ordinates are  $(4, -6)$



Since Max in plane shearing stress is equal to R

So  $\tau_{\max} = 4.05 \text{ ksi}$

Ans

$$\tan 2\theta = \frac{-2\tau_{xy}}{\sigma_x - \sigma_y} = \frac{-2(-6)}{4 + 0} = \frac{12}{4} = 3$$

$$\theta = 22.5^\circ$$

Pb 933

⑨

- ① PR-Stress    ② Max. in plane shear  
 ③ stress components at  $45^\circ$  &  $135^\circ$

①

$$\begin{aligned} \left. \begin{matrix} S_1 \\ S_2 \end{matrix} \right\} &= \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + (\tau_{xy})^2} \\ &= \frac{-60 + 60}{2} \pm \sqrt{\left(\frac{-60 - 60}{2}\right)^2 + (40)^2} \\ &= 0 \pm \sqrt{(-60)^2 + (40)^2} = \pm \sqrt{3600 + 1600} = \pm 72.11 \text{ MPa} \\ S_1 &= 72.11 \text{ MPa} \quad \& \quad S_2 = -72.11 \text{ MPa} \end{aligned}$$

②

$$\begin{aligned} \tau_{\max} &= \pm 72.11 \text{ MPa} \\ \tau_{\max} &= 72.11 \text{ MPa} \end{aligned}$$

③

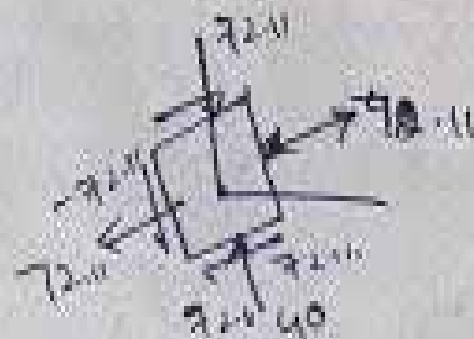
$$\begin{aligned} \sigma_{45^\circ} &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 2\theta + \tau_{xy} \sin 2\theta \\ &= \frac{-60 + 60}{2} + \frac{-60 - 60}{2} \cos 90^\circ + 40 \sin 90^\circ \\ &= 0 + 0 + 40 = 40 \text{ MPa} \\ \sigma_{135^\circ} &= 0 + \frac{-60 - 60}{2} \cos 270^\circ + 40 \sin 270^\circ \\ &= -40 \text{ MPa} \end{aligned}$$

④

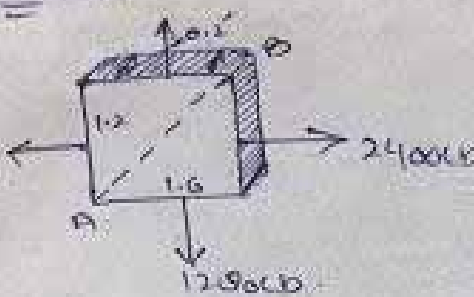
$$\begin{aligned} \tau_{45^\circ} &= \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta \\ &= \frac{-60 - 60}{2} \sin 90^\circ + 40 \cos 90^\circ \\ &= -60 \text{ MPa} \end{aligned}$$

$$\begin{aligned} \tau_{135^\circ} &= \frac{\sigma_x - \sigma_y}{2} \sin 2\theta + \tau_{xy} \cos 2\theta \\ &= \frac{-60 - 60}{2} \sin 270^\circ + 40 \cos 270^\circ \\ &= +60 \text{ MPa} \end{aligned}$$





Pb 3.4

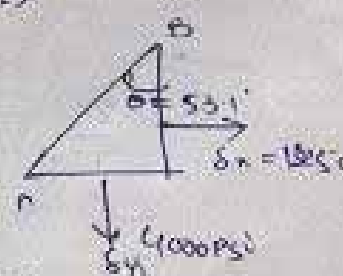


Reqd Stress component along AB

Since  $\sigma_x = \frac{P_x}{A_x} = \frac{2400}{1.6 \times 0.1} = 1500 \text{ PSI}$

$\sigma_y = \frac{1200}{1.2 \times 0.1} = 1000 \text{ PSI}$

and



$\tan \theta = \frac{1.6}{1.2} \Rightarrow \theta = \tan^{-1} 1.33$

$\theta = 53.1^\circ$

$$\sigma_x = \frac{2400}{1.2 \times 0.2} = 10 \text{ ksi}$$

$$\sigma_y = \frac{1200}{1.6 \times 0.2} = 4 \text{ ksi}$$

$$\begin{aligned} \sigma_{AB} &= \frac{\sigma_x + \sigma_y}{2} + \frac{\sigma_x - \sigma_y}{2} \cos 106.2^\circ \\ &= 7 + 3(-0.03) = 6.16 \text{ ksi} \end{aligned}$$

$$\tau_{AB} = \frac{\sigma_x - \sigma_y}{2} \sin 106.2^\circ = \frac{10 - 4}{2} \sin 106.2^\circ = 2.80 \text{ ksi}$$

• Prob 9.35

$$S_1 = \frac{S_x + S_y}{2} + \sqrt{\left(\frac{S_x - S_y}{2}\right)^2 + (Txy)^2}$$

$$S_2 = \frac{S_x + S_y}{2} - \sqrt{\left(\frac{S_x - S_y}{2}\right)^2 + (Txy)^2}$$

Adding ① & ②

$$S_1 + S_2 = S_x + S_y$$

$$2000 - 0000 = S_x + S_y$$

$$-6000 = S_x + S_y \rightarrow \text{①}$$

$$-6 = S_x + S_y$$

Now subtracting.

$$S_1 - S_2 = 2 \sqrt{\left(\frac{S_x - S_y}{2}\right)^2 + (S)^2}$$

$$2000 - 0000$$

$$2 + 0 = 2 \sqrt{\frac{(S_x - S_y)^2}{4} + 3^2}$$

$$10 = 2 \sqrt{\frac{(S_x - S_y)^2}{4} + 36}$$

$$10 = \sqrt{(S_x - S_y)^2 + 36}$$

$$100 = S_x - S_y + 36$$

$$100 - 36 = S_x - S_y$$

$$64 = S_x - S_y \rightarrow \text{②}$$

Adding ① & ②

$$64 = S_x - S_y$$

$$-6 = S_x + S_y$$

$$58 = 2S_x$$

$$S_x = 29 \text{ KSI}$$

$$64 = 29 - S_y$$

$$S_y = 29 - 64$$

$$S_y = -35 \text{ KSI}$$

PB Q41  $\sigma_1 = 50 \text{ ksi}$ ,  $\sigma_2 = 20 \text{ ksi}$

P Max. in Plane Shearing Stress

$$\tau = \frac{|\sigma_1 - \sigma_2|}{2} = \frac{50 - 20}{2} = 15 \text{ ksi}$$

§ Abs Max

$$\tau = \frac{|\sigma_1 - \sigma_2|}{2} = 15 \text{ ksi}$$

$$\tau_{\max} = \frac{|\sigma_1|}{2} = 25 \text{ ksi} \quad \text{or} \quad \tau_{\max} = \frac{|\sigma_2|}{2} = 10 \text{ ksi}$$

$$\text{So } \tau_{\max} = 25 \text{ ksi}$$

PB Q43  $\sigma_1 = 8 \text{ ksi}$ ,  $\sigma_2 = 2 \text{ ksi}$

Max in Plane Shearing Stress =  $\frac{|\sigma_1 - \sigma_2|}{2} = 1.5 \text{ ksi}$

$$(b) \tau_{\max} = \frac{|\sigma_1 - \sigma_2|}{2} = \frac{8 - 2}{2} = 1.5 \text{ ksi}$$

$$\tau_{\max} = \frac{|\sigma_1|}{2} = 2.5 \text{ ksi} \quad \text{or} \quad \tau_{\max} = \frac{|\sigma_2|}{2} = 1 \text{ ksi}$$

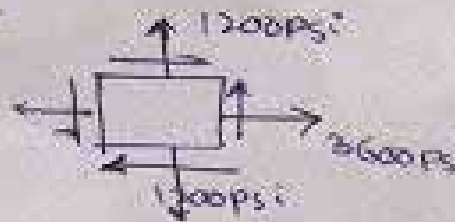
Prob 944

(a) Max in plane  $\tau_z = \frac{|\sigma_1 - \sigma_2|}{2} = \left( \frac{-40 + 0}{2} \right) = 20 \text{ MPa}$

(b)  $\tau_{\max} = \left| \frac{-40 + 0}{2} \right| = 20 \text{ MPa}$

$\tau_{\max} = \left| \frac{-40}{2} \right| = 20 \text{ MPa}$

$\tau_{\max} = \left| \frac{0}{2} \right| = 0 \text{ MPa}$

Pb 946

first of all finding  $\sigma_1$  &  $\sigma_2$

$$\begin{aligned}\frac{\sigma_1}{\sigma_2} &= \frac{3600 + 1200}{2} \pm \sqrt{\left(\frac{3600 - 1200}{2}\right)^2 + (1200)^2} \\ &= 2400 \pm \sqrt{1440000 + 1440000} \\ &= 2400 \pm 1697\end{aligned}$$

$$\sigma_1 = 2400 + 1697 = 4097 \text{ psi}$$

$$\sigma_2 = 703 \text{ psi}$$

so (a) Maxin plane Shearing stress

$$\tau = \frac{|\sigma_1 - \sigma_2|}{2} = \frac{4097 - 703}{2} = 1697 \text{ psi}$$

(b)

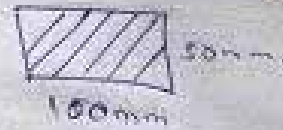
$$\tau_{avg} = \frac{|\sigma_1 - \sigma_2|}{2} = 1697 \text{ psi}$$

$$\tau_{\sigma_1} = \frac{|\sigma_1|}{2} = \frac{4097}{2} = 2048.5 \text{ psi}$$

$$\tau_{\sigma_2} = \frac{|\sigma_2|}{2} = \frac{703}{2} = 351.5 \text{ psi}$$

Pb 947-949 Same as above

Given  $E = 10 \times 10^9 \text{ Pa}$ ,  $S_{yp} = 30 \times 10^6 \text{ Pa}$   
 $F.O.S. = 2$ ,  $L = 2.5 \text{ m}$



- Req (a) Minimum length  
 (b)  $P_{all} = ?$

Sol

$$\frac{L_e}{R} = \sqrt{\frac{\pi^2 E}{S_{yp}}} = \sqrt{\frac{3.14^2 \times 10 \times 10^9}{30 \times 10^6}}$$

$$\frac{L_e}{R} = \sqrt{32065} = 57.32$$

$$I = \frac{100 \times 50^3}{12} = 1041666.67 \text{ mm}^4$$

$$A = 5000 \text{ mm}^2$$

$$I = AR^2 \Rightarrow R = \sqrt{\frac{I}{A}} = \sqrt{\frac{1041666.67 \text{ mm}^4}{5000 \text{ mm}^2}}$$

$$R = 14.43 \text{ mm}$$

$$\frac{L_e}{R} = 57.32 \Rightarrow L_e = 57.32 \times 14.43$$

$$L_e = 827.34 \text{ mm}$$

$$L_e = \frac{1}{2} L$$

$$L = 2 \times L_e = 1654.6 \text{ mm} = 1.65 \text{ m}$$

(b)

$$P_{cr} = \frac{\pi^2 EI}{L_e^2}$$

$$P_{all} = \frac{P_{cr}}{F.O.S.}$$

$$P_{cr} = F.O.S. \times P_{all} \quad \text{f} \quad L_e = \frac{L}{2} = 1.25 \text{ m}$$

So

$$2 \times P_{all} = \frac{\pi^2 EI}{(1.25)^2} = \frac{3.14^2 \times 10 \times 10^9 \times 1041666.67}{1.5625}$$

$$P_{all} = \frac{3.14^2 \times 10 \times 10^9 \times 1041666.67}{3.125} = 32065.1 \text{ N} = 32.06 \text{ kN}$$

Pb 1103

$$A = 3 \times 2 = 1.5 \text{ in}^2$$

$$L = 6 \text{ ft} = 72 \text{ in}$$

$$E = 10.3 \times 10^6 \text{ psi}$$

$$F.S. = 2$$

$$Req. P_{safe} = ?$$

$$\underline{\text{Sol}} \quad P_{cr} = \frac{\pi^2 EI}{L^2}$$

$$= 2.44$$

For Hinged ends.

$$I = \frac{2 \times 3^3}{12} = 0.5$$

$$K = \frac{L}{\sqrt{A}} =$$

$$\text{So } P_{cr} = \frac{3.14^2 \times 10.3 \times 10^6 \times 0.5 \text{ in}^4}{(72)^2 \text{ in}^2}$$

$$P_{cr} = 9794.95 \text{ lb}$$

FOR Fixed ends

$$L_e = \frac{L}{2} = 36 \text{ in}$$

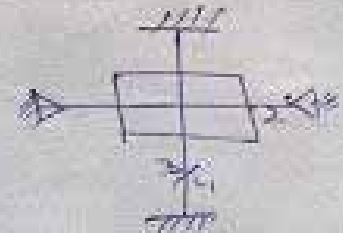
$$I = \frac{\left(\frac{3}{4}\right)^3 \times 2}{12} = 0.070$$

So

$$P_{cr} = \frac{\pi^2 EI}{(L_e)^2} = \frac{(3.14)^2 \times (10.3 \times 10^6) \times 0.070}{(36)^2}$$

$$P_{cr} = 5405.16$$

$$P_{all} = \frac{P_{cr}}{F.S.} = \frac{5405.16}{2} = 2702.58 \text{ lb}$$



MASOOD  
AKHTAR  
2007  
B#3



Pb 1101 Given  
 $P_{cr} = 20 \times 10^3 \text{ lb}$   $L = 10' = 120''$   
 $E = 29 \times 10^6 \text{ psi}$   
Req  $b = ?$   $Le = L$   
 Since  $P_{cr} = \frac{\pi^2 EI}{Le^2}$   
 $I = \frac{P_{cr} \times Le^2}{\pi^2 E} = \frac{20 \times 10^3 \times (120)^2}{3.14^2 \times 29 \times 10^6} = 1.0072 \text{ in}^4$   
 $I = \frac{b^4}{12} = \frac{b^4}{12} \Rightarrow b^4 = I \times 12 = 1.0072 \times 12 \Rightarrow b = 1.86''$

---

Pb 1105  
 $I = \frac{P_{cr} \times Le^2}{\pi^2 E} = \frac{20 \times 10^3 \times (120)^2}{3.14^2 \times 1.6 \times 10^6}$   
 $I = 10.85$   
 $I = \frac{b^4}{12} \Rightarrow b^4 = I \times 12 = 10.85 \times 12 \Rightarrow b = 3.04 \text{ in}$

Pr 1106

$C_{40 \times 45}$

Given  $\rightarrow$   $A = 67.3 \text{ mm}$

$I = 67.3 \times 10^6 \text{ mm}^4 = 10^6 \times 10^{-12} = 10^{-6} \text{ m}^4$

$r = 109 \text{ mm}$ ,  $E = 200 \times 10^9 \text{ Pa}$

$S_{yp} = 240 \times 10^6 \text{ Pa}$

① minimum length  $= L = ?$

②  $P_{all}$  when  $L = 12 \text{ m}$  &  $F.S = 2.5$

Sol


we know that

$$\frac{L_e}{R} = \sqrt{\frac{\pi^2 E}{S_{yp}}} = \sqrt{\frac{3.14^2 \times 200 \times 10^9}{240 \times 10^6}} = 90.64$$

$$L_e = 90.64 \times R = 9000.19 \text{ mm}$$

$$L = 9.000 \text{ m}$$

③



$$P_{cr} = \frac{\pi^2 EI}{L_e^2} \quad (L_e = L)$$

$$P_{all} = \frac{P_{cr}}{F.S} \Rightarrow P_{cr} = P_{all} \times F.S$$

$$P_{all} \times F.S = \frac{\pi^2 EI}{L_e^2}$$

$$P_{all} = \frac{3.14^2 \times 200 \times 10^9 \times 67.3 \times 10^{-6}}{2.5 \times (9)^2}$$

$$P_{all} = 727 \text{ kN}$$

Prob 1110

Given:  $S_{yp} = 200 \text{ MPa}$ ,  $E = 200 \text{ GPa}$

(a) Hinged ends,  $L = 9 \text{ m}$

(b) built-in and  $L = 10 \text{ m}$

(c) built-in  $\&$  free braced at mid

Req  $P_{cr} = ?$

sol  $A = 1550 \text{ mm}^2$   $I = 2.657 \times 10^8 \text{ mm}^4$   
 $r = 15.2 \text{ mm}$   $r_2 = 63 \text{ mm}$

(a) 
$$C_c = \sqrt{\frac{2\pi^2 E}{S_{yp}}} = 101.09$$

$$\frac{L_e}{r} = \frac{9000}{63} = 142.85$$

Since  $C_c < \frac{L_e}{r}$  So

$$S_c = \frac{12\pi^2 E}{23 \left(\frac{L_e}{r}\right)^2} = \frac{12 \times 2.14 \times 200 \times 10^9}{23 (142.85)^2}$$

$$= 18.05 \text{ MPa} \quad 50.4 \text{ MPa}$$

Sub to  $P = S_c A$

$$= 18.05 \text{ MPa}$$

$$= 50.4 \times 10^6 \times 1550 \times 10^{-6} \text{ m}^2$$

$$P = 78200 \text{ N}$$

(b) Fixed ends  $L = 10 \text{ m}$

So  $L_e = 0.7L = 7 \text{ m}$

$$\frac{L_e}{r} = \frac{7000}{63} = 111.11$$

$$L_e = 0.5L = 5 \text{ m}$$

$$\frac{L_e}{r} = \frac{5000}{63} = 79.2$$

Since  $C_c > \frac{L_e}{r}$  So

$$S_{w2} = \left( 1 - \frac{(L/r)^2}{2C_c^2} \right) \frac{S_y}{F.S}$$

$$F.S = \frac{5}{3} + 3 \frac{(L/r)}{C_c} - \frac{(L/r)^3}{C_c^3}$$

$$= \frac{5}{3} + 3 \frac{(99.3)}{81.07} - \frac{(99.3)^3}{81.07^3}$$

$$= 1.66 + 0.291 - 0.050$$

$$F.S = 1.892 = 1.90$$

So

$$S_{w2} = \left( 1 - \frac{(99.3)^2}{2(81.07)^2} \right) \frac{300 \times 10^6 \text{ Pa}}{1.90}$$

$$= (0.69) \times 200 \text{ MPa}$$

$$= 138 \text{ MPa}$$

$$P_{safe} = S_{w2} A = 138 \times 10^6 / 155 \times 10^{-6}$$

$$= 213 \text{ kN}$$

$$= 213000 \text{ N}$$

②  $L = 5 \text{ m}$ ,  $C_c = 0.7L = 0.7 \times 5 = 3.5 \text{ m}$

$$\frac{L}{r} = \frac{3500}{63} = 55.55$$

Since  $C_c > L/r$  So

$$S_{w2} = \left( 1 - \frac{(L/r)^2}{2C_c^2} \right) \frac{S_y}{F.S}$$

$$F.S = \frac{5}{3} + 3 \frac{(L/r)}{C_c} - \frac{(L/r)^3}{C_c^3}$$

$$= 1.66 + 0.20 - 0.02 = 1.84$$

So  $S_{w2} = \left( 1 - \frac{(55.55)^2}{2(101.07)^2} \right) \times \frac{300 \text{ MPa}}{1.84}$

*Handwritten notes on the right margin:*  
 $S_y = 0.05 \times 300 \text{ MPa}$   
 $S_y = 15 \text{ MPa}$   
 $P = 211500 \text{ N}$

Prob 112 Given length  $= L = 5m$   
 Req. Find  $le/r$  if  
 (a) Circular section of 80mm radius  
 (b) Square section.

Sol  $le/r$  is a slender member.  
 (a)  $le = \frac{L}{2} = 2.5m$   
 $r = \sqrt{\frac{I}{A}} = \sqrt{\frac{\pi r^4}{4\pi r^2}} = \frac{r}{2} = 40mm$   
 So  $\frac{le}{r} = \frac{2500}{40} = 62.5$

(b)  $le = \frac{L}{2} = 2.5m = 2500mm$   
 $r = \sqrt{\frac{I}{A}} = \sqrt{\frac{b^4}{12b}} = \sqrt{\frac{b^3}{12}} = 14.4$   
 So  $\frac{le}{r} = \frac{2500}{14.4} = 173.6$

Prob 113 Given  $L = 12ft = 144in$   
 one end fixed & other hinged.

Req  $le/r$  if  
 (a) Circular with 2in rad.  
 (b) 2 in square.

Sol  $le = 0.7L = 0.7 \times 144 = 100.8in$   
 $r = \frac{1}{2} = 1$   
 $\frac{le}{r} = 100.8in$

(b)  $r = \sqrt{\frac{b^4}{12b}} = \sqrt{\frac{2^3}{12}} = 0.72$   
 $\frac{le}{r} = \frac{100.8}{0.72} = 140$

Pb 11.14 Given  $W_{250 \times 167}$  hinged ends,  
 $P = 1600 \text{ kN}$   
 $S_{yp} = 300 \times 10^6 \text{ Pa}$ ,  $E = 200 \text{ GPa}$   
 $A = 21300 \text{ mm}^2$ ,  $r = 69.1 \text{ mm}$ ,  $I = 300 \times 10^8 \text{ mm}^4$   
Req Length = ?

sol To find length we have to find  $l_e$  first

$$\frac{l_e}{R} = \sqrt{\frac{2\pi^2 E}{S_{yp}}}$$

$$l_e = \sqrt{\frac{2\pi^2 E}{S_{yp}}} = 101.07$$

$$\frac{l_e}{R} = \sqrt{\frac{2\pi^2 E}{S_{yp}}} \quad S_{yp} = \frac{P}{A} = \frac{1600 \times 10^3}{21300 \times 10^{-6}} = 75117370.89$$

$$S_{yp} = \frac{12\pi^2 E}{23 \left(\frac{l_e}{R}\right)^2}$$

$$\left(\frac{l_e}{R}\right)^2 = \frac{12 \times 2.14^2 \times 200 \times 10^9}{23 \times 75117370.89}$$

$$(l_e)^2 = 12 \times 12696.22 \times R^2$$

$$= 633$$

$$l_e = 79.69.0$$

$$L = 7969.8 \text{ mm}$$

$l_e < \frac{l_e}{R}$



Pb.1115 Given  $W_{14 \times 12}$   
 $L = 30'$ ,  $S_y = 50 \text{ ksi}$ ,  $E = 29 \times 10^6 \text{ psi}$   
 $A = 24.1 \text{ in}^2$ ,  $I = 99.2 \text{ in}^4$ ,  $r = 2.48$   
Req  $P_{\text{safe}} = ?$   
Sol  

$$C_c = \sqrt{\frac{\pi^2 E}{S_y}} = 100.94 = 103$$

$$\frac{L_e}{R} = \frac{30 \times 12}{2.48} = 145.16$$
Since  $C_c < L_e/R$   

$$S_y \quad S_w = \frac{12 \pi^2 E}{23 (L_e/R)^2}$$

$$S_w = \frac{12 \times 3.14^2 \times 29 \times 10^6 \text{ psi}}{23 (145.16)^2}$$

$$S_w = 7079.75 \text{ psi}$$

$$P_{\text{safe}} = 170.6 \text{ kips}$$

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Pb.1116 Given  $W_{21 \times 55}$  Fixed ends.  
Req (a)  $P_{\text{safe}}$  (b)  $S_y = 250 \text{ MPa}$   
 $L = 10 \text{ m}$ ,  $E = 200 \text{ GPa}$   
 $A = 6620 \text{ mm}^2$ ,  $r = 39.3 \text{ mm}$   
Sol  

$$C_c = \sqrt{\frac{\pi^2 E}{S_y}} = 125.6$$

$$R = 39.3 \text{ mm}$$

$$\frac{L_e}{R} = \frac{10000}{39.3} = 254.45$$
Since  $C_c < L_e/R$   

$$S_y \quad P = S_w \times A$$

$$S_w = \frac{12 \pi^2 E}{23 (L_e/R)^2} = 15090 \text{ MPa}$$

$$P = 15090 \times 6620 \times 10^{-6} = 105909.6 \text{ N}$$

Pb 1110 Given  $C_{50 \times 45}$   
 $L = 10m$ ,  $E = 200 \times 10^9 Pa$   
 $E_{yp} = 300 \times 10^6 Pa$   
 $A = 5670 mm^2$ ,  $r = 16.8 mm$

sol Req  $P = ?$

$$C_c = \sqrt{\frac{A^2 E}{E_{yp}}} = \sqrt{\frac{2 \times 3.14^2 \times 200 \times 10^9}{300 \times 10^6}}$$

$$C_c = 101.075$$

$$\frac{L}{r} = \frac{10000}{16.8} = 595.23$$

$$C_c < \frac{L}{r} \quad 30$$

$$S_w = \frac{12 A^2 E}{23 (L/r)^2} = \frac{12 \times 3.14^2 \times 200 \times 10^9}{23 (595.23)^2}$$

$$S_w = 337281523.0 \text{ Pa} \quad 2903842.64$$

$$P = S_w \times A = 337281523.0 \times 5670 \times 10^{-6}$$

$$= 2903842.64 \text{ N} = 2.90 \text{ MN}$$

$$= 16464.2 \text{ kN}$$


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Pb 1120 Given  $S_{50 \times 50}$

①  $L = 1m$     ②  $L = 2m$   
 $A = 6650 mm^2$ ,  $r = 25 mm$

Req  $P_{safe} = ?$

$$\frac{L}{r} = \frac{L}{r} = \frac{1000}{25} = 40$$

Since  $\frac{L}{r} \quad 12 < \frac{L}{r} < 55$

SO  $S_w = (212 - 1.59(\frac{L}{r})) MPa$   
 $S_w = 140.4 MPa$   
 $P = S_w \times A$   
 ①  $P = 906 kN$



(n)  $\frac{L}{R} = \frac{3600}{30} = 120$   
 $\frac{L}{R} > 55$  so  
 $s_w = \frac{372 \times 10^3}{\left(\frac{L}{R}\right)^2} = 25.03 \text{ MPa.}$   
 $P = s_w \times A = 25.03 \times 10^6 \times 6650 \times 10^{-6}$   
 $= \underline{\underline{171 \text{ kN}}}$

Pb 11.29 Given (S1, S2)  
 (a)  $L = 4 \text{ ft}$  (b)  $L = 10 \text{ ft}$   
 $A = 10.3 \text{ in}^2$ ,  $v = 0.980 \text{ in}$   
Req.  $P_{\text{max}} = ?$

Sol  
 (a)  $\frac{L}{R} = \frac{4 \times 12}{0.980} = 48.9$   
 $12 < \frac{L}{R} < 49.055$  so  
 For 02 - Cow Term  
 $s_w = (30.7 - 0.29 \left(\frac{L}{R}\right)) \text{ Ksi}$   
 $(30.7 - 0.29 \times 48.9) \text{ Ksi}$   
 $s_w = 19.43 \text{ Ksi}$   
 $P = 19.43 \times 10^3 \times 10.3 = 200 \text{ kips}$

(b)  $\frac{L}{R} = \frac{10 \times 12}{0.980} = 122.4$   
 so  
 $s_w = \frac{54000}{\left(\frac{L}{R}\right)^2} = \frac{54000}{(122.4)^2} = \frac{54000}{14975.76}$   
 $s_w = 3.60 \text{ Ksi} \Rightarrow P = s_w \times A$   
 $P = 37.0 \text{ kips}$

Pb 1131

150 mm x 200 mm

(a) 2m (b) 4m  $E = 11.5 \times 10^9 \text{ Pa}$

So

$$S_w = \frac{0.3 E}{\left(\frac{L}{d}\right)^2} = \frac{0.3 \times 11.5 \times 10^9}{\left(\frac{2}{0.15}\right)^2}$$

$$S_w = 19406250$$

$$P_{safe} = S_w \times A = (19406250 \times (0.2 \times 0.15))$$

$$= 582 \text{ kN}$$

(b)

$$S_w = \frac{0.3 E}{\left(\frac{L}{d}\right)^2} = \frac{0.3 \times 11.5 \times 10^9}{\left(\frac{4}{0.15}\right)^2}$$

$$S_w = 9051562.5 \text{ Pa}$$

$$P = 148 \text{ kN}$$

Pb 1132

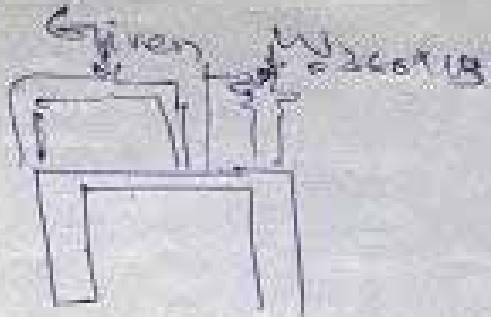
(a) 2 x 8 in (b) 12 ft  $E = 1.6 \times 10^6 \text{ psi}$   $A = 16 \text{ in}^2$

(a)  $d = 8 \text{ in}$

$$S_w = \frac{0.3 \times 1.6 \times 10^6}{\left(\frac{8 \times 12}{2}\right)^2} = 370.3 \text{ psi}$$

$$P = S_w \times A = 5925.9 = \underline{5926 \text{ lb}}$$

PD 1133



Given  $W 300 \times 134$

$P_0 = 400 \text{ kN}$ ,  $S_y = 250 \text{ MPa}$ ,  $E = 200 \text{ GPa}$

$A = 7100 \text{ mm}^2$ ,  $S_x = 2230 \times 10^3 \text{ mm}^3$ ,  $r = 94 \text{ mm}$

Sol

$$\frac{L_e}{r} = \frac{7000}{94} = 74.46$$

$$C_c = \sqrt{\frac{2\pi^2 E}{S_y}} = 125.6$$

$C_c > L_e$ , so

$$F_{cr} = \left(1 - \frac{(L_e/r)^2}{C_c^2}\right) \frac{S_y}{F.S.}$$

$$F.S. = \frac{S_y}{3} + \frac{3(L_e/r)^2}{8C_c^2} - \frac{(L_e/r)^4}{8C_c^4}$$

$$= \frac{250}{3} + \frac{3(74.46)^2}{8 \times 125.6^2} - \frac{(74.46)^4}{8 \times (125.6)^4}$$

$$= 166.6667 + 0.22 - 0.02$$

$$F.S. = 1.06$$

$$\text{So } S_w = \left(1 - \frac{(L_e/r)^2}{2C_c^2}\right) \frac{250}{1.06} = 110 \text{ MPa}$$

Now

$$S_{we} = \frac{2P}{A} + \frac{M}{S}$$

$$110.7 \text{ MPa} = \left( \frac{400 \times 10^3 + P}{17100 \times 10^{-6} \text{ m}^2} \right) + \left( \frac{P \times 0.125 - 400 \times 10^3 \times 0.075}{2320 \times 10^{-6} \text{ m}^3} \right)$$

$$= \left( \frac{400000 + P}{17100 \times 10^{-6}} \right) + \left( \frac{0.05P - 30000}{2320 \times 10^{-6}} \right)$$

$$110.7 \times 10^6 = 23391812.07 + 0.000504777P \times 10^6$$

$$50.47P + 5364P - 12075$$

$$110.7 \times 10^6 = 23378937.37 + 112.11P$$

$$112.11P = 110.7 \times 10^6 - 23378937.37$$

$$P = \underline{\underline{093 \text{ kN}}}$$

Prob 1134

W360 x 122

Given:  $L_e = 10 \text{ m}$ ,  $S_{yp} = 290 \text{ MPa}$ ,  $E = 200 \text{ GPa}$

$A = 15500 \text{ mm}^2$ ,  $S_x = 2010 \times 10^3 \text{ mm}^3$ ,  $r = 63 \text{ mm}$

$E = 200 \text{ GPa}$

REQ  $P_c = ?$

Sol

$$\frac{L_e}{r} = \frac{10 \times 10^3}{63} = 158.7$$

$$C = \sqrt{\frac{2\pi^2 E}{S_{yp}}} = 116.6$$

$C < \frac{L_e}{r}$  So

$$S_{N2} = \frac{12\pi^2 E}{23 \left( \frac{L_e}{r} \right)^2} = 40849697.81 \text{ Pa}$$

Now

$$S_w = \frac{P}{A} + \frac{M}{S_x}$$

$$= \frac{P_c}{15500 \times 10^{-6} \text{ m}^2} + \frac{P_c \times 300 \times 10^{-3} \text{ m}}{2010 \times 10^{-6} \text{ m}^3}$$



$$= P_c (64.51 + 149.2)$$

$$40049697.7 \frac{\text{N}}{\text{m}^2} = 213.71 P_c$$

$$P_c = 191145.4 \text{ N} = 191 \text{ kN.}$$

Pb 135 Same as above just put  $l_e = 4.5 \text{ m}$  & find  $S_w$  then  $P_c$ .

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Pb 136 Given:  $l_e = 5 \text{ ft} = 60 \text{ in}$   

$E = 5 \text{ in, } S_y = 36 \times 10^6 \text{ psi}$   
 $P_o = 11 \times 10^3 \text{ lb, } E = 29 \times 10^6 \text{ psi}$

So Req  $P_c = ?$

$$\frac{l_e}{r} = \frac{60}{0.577} = 103.98 \checkmark$$



$$\begin{aligned}
 c_c &= \sqrt{\frac{271E}{s_{yp}}} = \sqrt{\quad} = 126.03 \\
 \text{Since } c_c &> l_{c1} \text{ so} \\
 \text{So } S_w &= \left(1 - \frac{(l_{c1})^2}{c_c^2}\right) \frac{s_{yp}}{F.S} \\
 F.S &= \frac{\frac{3}{8} \frac{s}{3} + \frac{3(l_{c1})}{8c_c} - \frac{(l_{c1})^3}{8c_c^3}}{1} \\
 &= 1.66 + 0.309 - 0.07 \\
 F.S &= 1.89 \\
 \text{So } S_w &= \frac{s_{yp}}{F.S} \left(1 - \frac{(l_{c1})^2}{c_c^2}\right) \frac{26 \times 10^3 \text{ psi}}{1.89} \\
 S_w &= 12.56 \text{ KSI} \\
 \text{So } S_w &= \frac{EP}{A} + \frac{M}{S_x} \\
 &= \left(\frac{11 \times 10^3 + P_e}{6}\right) + \left(\frac{P_e \times 5}{3}\right) \\
 S_x &= \frac{I}{c} = \frac{bh^3}{12} = \frac{2 \times 27}{12} = 3 \text{ in}^3 \\
 \text{So } S_w &= \left(\frac{11 \times 10^3 + P_e}{6}\right) + \left(\frac{P_e \times 5}{3}\right) \\
 12.56 \times 10^3 &= 1833.33 + 0.16 P_e = 1.66 P_e \\
 " &= " + 1.62 P_e \\
 1.01 P_e &= \\
 P_e &= 5093.7 \text{ N}
 \end{aligned}$$

PD 1137      Given       $L = 8 \text{ ft}$   
 $L_e = 2L = 16 \text{ ft}$   
 $e = 2 \text{ ft} = 24 \text{ in}$   
 $d_o = 4 \text{ in}$        $A = 3.14 \text{ in}^2$   
Req weight of sign  $P = ?$

Sol

$\frac{L_e}{R} = \frac{16 \times 12}{R}$       Find  $R$

$R = \sqrt{\frac{I}{A}} = \sqrt{\frac{7.231}{3.14}} = 1.50$

so  $\frac{L_e}{R} = \frac{16 \times 12}{1.50} = 128$

$C_c = \sqrt{\frac{2\pi^2 E}{S_{yp}}} = 106.94$

$C_c < \frac{L_e}{R}$

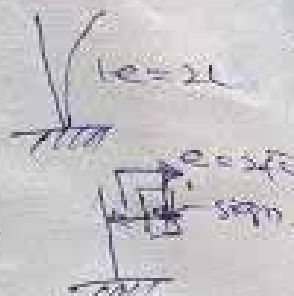
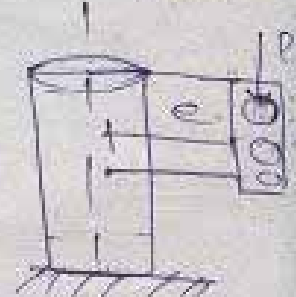
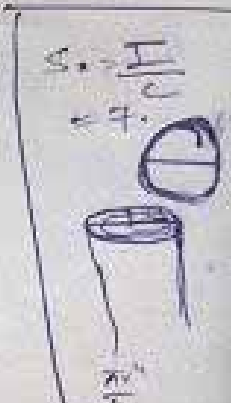
so  $S_w = \frac{12\pi^2 E}{23(L_e/r)^2} = 9105.22 \text{ psi}$

$S_w = \frac{EP}{A} + \frac{M}{S_x} \rightarrow \text{①}$

$S_x = \frac{I}{c} = \frac{\pi r^4}{4} = \frac{\pi r^4}{4r} = \frac{\pi r^3}{4}$   
 $= \frac{\pi r^3}{4} = \frac{3.14 \times (2.25)^3}{4}$   
 $= 0.94 \text{ in}^3$

so From eq ①

$9105.22 = \left( \frac{P \times 24}{3.14} \right) + \left( \frac{P \times 24}{0.94} \right)$        $\Rightarrow P = 3035.09 \text{ lb}$   
 $= 0.31 P + 2.60 P$   
 $\Rightarrow 2P = 9105.22$

Pb 1130

W260x134

Given  $L_e = 6m$

$P_1 = 260 kN$  &  $P_2 = 220 kN$

$S_{yp} = 250 MPa$  &  $E = 200 GPa$

$A = 17100 mm^2$  &  $S_x = 2330 \times 10^3 mm^3$

$r = 94 mm$

Sol

Req  $\Rightarrow e = ?$

$$\frac{L_e}{r} = \frac{6 \times 1000}{94} = 63.82$$

$$C_c = \sqrt{\frac{2\pi^2 E}{S_{yp}}} = \sqrt{\frac{2\pi^2 \times 200 \times 10^9}{250 \times 10^6}} = 125.6$$

$C_c > \frac{L_e}{r}$  So

$$F.S. = \left(1 - \frac{(L_e/r)^2}{C_c^2}\right) \frac{S_{yp}}{F.S.}$$

$$F.S. = \frac{5}{3} + \frac{3(L_e/r)}{C_c} - \frac{(L_e/r)^3}{C_c^3}$$

$$= 1.66 + \frac{3(63.82)}{125.6} - \frac{(63.82)^3}{125.6^3}$$

$$F.S. = 1.66 + 1.54 - 0.016 = 1.03$$

So  $S_w = \left(1 - \frac{(L_e/r)^2}{C_c^2}\right) \frac{S_{yp}}{F.S.}$

$$S_w = 118.97 MPa$$

$$S_w = \frac{P}{A} + \frac{M}{S_x}$$

$$= \left(\frac{P_1 + P_2}{A}\right) + \left(\frac{M}{S_x}\right) = \left(\frac{260 \times 10^3 + 220 \times 10^3}{17100 \times 10^{-6}}\right)$$

$$+ \frac{220 \times 10^3 \times e}{2330 \times 10^6} \Rightarrow$$



$$110.97 \text{ MPa} = 20070175.94 + 94420600.06 e$$

$$\frac{110.97 \times 10^6 - 20070175.94}{94420600.06} = e$$

$$e = 0.96 \text{ m} = \underline{\underline{962 \text{ mm}}}$$
  

Pb 1139      Q. 2.45

Given:  $L = 2.2 \text{ m}$ ,  $P = 50 \text{ kN}$ ,  
 $S_y = 300 \text{ MPa}$ ,  $S_x = 140 \text{ MPa}$ ,  
 $E = 200 \text{ GPa}$ ,  $A = 5690 \text{ mm}^2$ ,  
 $I = 67.33 \times 10^6 \text{ mm}^4$ ,  $r = 19.3 \text{ mm}$ ,  
 $S_x = 442 \times 10^3 \text{ mm}^3$ ,  $S_y = 33.6 \times 10^3 \text{ mm}^3$

Reqd  $e = ?$        $S_x = P/A \Rightarrow 140 \times 10^6 \times 5690 \times 10^{-6} = P$   
 $P = 796600 \text{ N}$

Sol ( $e = 2.2 \text{ m}$ )       $\frac{e}{r} = \frac{2.2 \times 1000}{19.3} = 113.9$

$C_L = \sqrt{\frac{2\lambda^2 E}{S_y}} = 101.07$

$e < \frac{L}{r}$       so

$S_w = \frac{12\lambda^2 E}{22 \left(\frac{L}{r}\right)^2} = 79192744.95 \text{ Pa}$

$S_w = \left( \frac{P}{A} + \frac{M}{S_y} \right) = \frac{50 \times 10^3}{5690 \times 10^{-6}} + \frac{50 \times 10^3 \times e}{442 \times 10^3}$

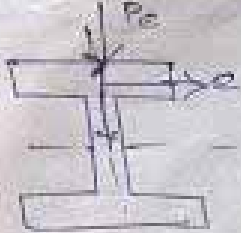
$79192744.95 = \frac{50 \times 10^3}{5690 \times 10^{-6}} + \frac{50 \times 10^3 \times e}{442 \times 10^3}$

$79192744.95 = 8787346.21 + 113.22719 e$

$\times \quad e = 0.622 \text{ m}$       ?

Prob 11.41  
W14x90 Given  $L = 30 \times 12 = 360 \text{ in}$

$P_o = 65 \times 10^3 \text{ lb} \Rightarrow P_c = 90 \times 10^3 \text{ lb}$  on  $90 \times 10^3$   
 $S_{yp} = 50 \text{ ksi}$   $E = 29 \times 10^6 \text{ psi}$   
 $A = 26.5 \text{ in}^2$ ,  $I = 999 \text{ in}^4$   
 $S_x = 143 \text{ in}^3$ ,  $S_y = 49.9 \text{ in}^3$   
 $r_y = 3.70 \text{ in}$ ,  $r_x = 6.14 \text{ in}$



$\frac{L_e}{r} = \frac{360}{3.70} = 97.29$

$C_c = \sqrt{\frac{2\pi^2 E}{S_{yp}}} = 106.94$

$C_c > \frac{L_e}{r}$  so

$F.S. = \frac{5}{16} + \frac{3 \left( \frac{L_e}{r} \right)}{C_c} - \frac{\left( \frac{L_e}{r} \right)^2}{C_c^2}$

$F.S. = 1.66 + 0.34 - 0.054 = 1.906$

$S_w = \left( 1 - \frac{\left( \frac{L_e}{r} \right)^2}{2C_c^2} \right) \frac{S_{yp}}{F.S.}$

$S_w = 15.4 \text{ ksi}$

so  $S_w = \frac{E P_o}{A} + \frac{M}{S_x}$

$= \frac{65 \times 10^3}{26.5} + \frac{90 \times 10^3 \times 6.14}{143}$

$\frac{15.4 \times 10^3}{15425.42} = 5049.05 + 629.32$

$e = 15.21 \text{ in}$

$\frac{y}{r} = 20.10$ 
 $\frac{y}{r} = 20.1$

$$F.S = 1.66 + \frac{3(20.10)^2}{8 \times 101.87} - \frac{(20.10)^3}{8 \times (101.87)^2}$$

$$= 1.66 + 1.499 - 0.000971$$

$$= 3.15$$

So
 
$$S_{xx} = \left( 1 - \frac{(20.10)^2}{2(101.87)^2} \right) \times \frac{300 \times 10^6}{3.15}$$

$$S_{xx} = 418267950.2 \text{ Pa}$$

$$= \frac{P}{A} + \frac{My}{I_{xx}}$$

$$148787346.2 +$$

$$\frac{bh^3}{12}$$

$$\frac{bh^3}{12}$$

$$\frac{bh^3}{12} + b \times y_g \left( \frac{h}{4} \right)$$

$$\frac{bh^3}{12} + \frac{bh^3}{32}$$

$$\frac{bh^3}{96} + \frac{3bh^3}{96}$$

$$\frac{4bh^3}{96}$$

$$I_x = \frac{bh^3}{24}$$

$$S = \frac{I}{c}$$

$$I_x =$$

$$\frac{I_x}{c} = \frac{\frac{bh^3}{24}}{\frac{h}{4}} = \frac{bh^3}{24} \times \frac{4}{h}$$

$$\frac{bh^2}{6}$$

$$S = \frac{I}{c}$$

$$Z = \frac{I}{c}$$

$$S = \frac{I}{c}$$

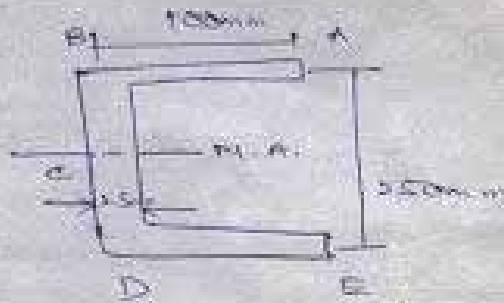
Pb 1321

Given  $V = 2000 \text{ N}$

Find  $\tau$  at  $y = 0$  & draw it

① Shear stress  $\tau = \frac{VQ}{I}$

Sol



Q1

Since  $Q = \frac{VQ}{I}$

First of all finding  $I$

$$I = \sum \left( \frac{bh^3}{12} + Ad^2 \right)$$

$$= 2 \left( \frac{100 \times 12.5^3}{12} + (100 \times 12.5)(125)^2 \right) + \left( \frac{250 \times 12.5^3}{12} + (250 \times 12.5)(0)^2 \right)$$

$$= 2(130.2 + 3906150) + (3255208.3)$$

$$1106796.873 \text{ mm}^4$$

$$1106796.873 \times 10^{-12}$$

$$11.06 \times 10^{-6} \text{ m}^4$$

$$I = 11.06 \times 10^{-6} \text{ m}^4$$

Now for  $Q_{1-1}$

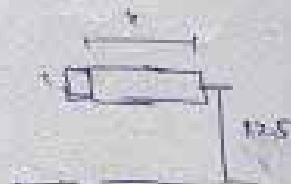
$$Q_{1-1} = A'y' = 2.52 \times 12.5$$

$$= 312.5 \text{ mm}^2$$

$$\tau_{1-1} = \frac{VQ_{1-1}}{I} = \frac{2000 \times 312.5 \times 10^{-6}}{11.06 \times 10^{-6}} = 56.45 \text{ N/mm}^2$$

$$\text{At } x=0 \quad \tau_{1-1} = 0 \quad 56.45 \text{ N/mm}^2$$

$$\text{At } x=+1 \quad \tau_{1-1} = 56.45 \text{ N/mm}^2$$



Now  $Q_{x-x}$

so  $Q_{x-x} = Q_{A1} + Q_{A2}$

$= A_1 y_1 + A_2 y_2$

$= 100 \times 2.5 \times 125 + 12.9 \times \left(\frac{2.5^2}{2}\right) \times \left(125 - \frac{2.5}{2}\right)$

$Q_{x-x} = 31250 + 312.5y - 1.25y^2$

$Q_{x-x} = 0$

$Q_{x-x} = \frac{V Q_{x-x}}{I} = \frac{2000 \times (31250 + 312.5y - 1.25y^2)}{11.06 \times 10^6 \text{ mm}^4}$

$2000y = 0$   $Q_{x-x} = \frac{2000 \times (31250 + 312.5 \times 0 - 1.25 \times 0)}{11.06 \times 10^6 \text{ mm}^4}$

$= 5.650 \text{ N/mm}^2$

so  $y = 125$

$Q_{x-x} = \frac{2000 \times (31250 + 312.5 \times 125 - 1.25 \times 125^2)}{11.06 \times 10^6 \text{ mm}^4}$

$= 9.182 \text{ N/mm}^2$

so

$5.645 \text{ N/mm}^2$

$9.182 \text{ N/mm}^2$

$5.645 \text{ N/mm}^2$

For  $e = \frac{b^3 h^3}{4I}$

$= \frac{100^3 \times 250^3 \times 125}{4 \times 11.06 \times 10^6}$

$= 35.3 \text{ mm}$

Pb 1322      GIVEN       $t_1 = t_2 = t_3 = 10 \text{ mm}$   
 $h_1 = 100 \text{ mm}$ ,       $h_2 = 140 \text{ mm}$   
 $b_3 = 200 \text{ mm}$

REQ      Shear Centre =  $P_0$

Sol      sin c for this section

$$\frac{b_1}{b_2} = \frac{I_2}{I_1}$$

so  $I_1 = \frac{bh^3}{12} = \frac{10 \times 100^3}{12} = 4860000 \text{ mm}^4$

$$I_2 = \frac{bh^3}{12} = \frac{10 \times 140^3}{12} = 2286666.667 \text{ mm}^4$$

∴  $b_1 + b_2 = 210 \text{ mm}$   
 $b_2 = (210 \text{ mm} - b_1)$

so

$$\frac{b_1}{(210 \text{ mm} - b_1)} = \frac{2286666.667}{4860000}$$

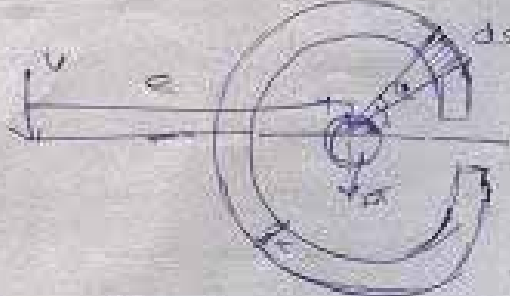
$$4860000 b_1 = 480.2 \times 10^6 \text{ mm} - 2286666.667 b_1$$

$$7146666.667 b_1 = 480.2 \times 10^6 \text{ mm}$$

so  $b_2 = 210 - 67.19 = 142.81 \text{ mm}$        $b_1 = 67.19 \text{ mm}$

Ph 1323

Given  $\Rightarrow$  radius =  $r$ , thickness =  $t$   
Reqd  $e = ?$



$ds = r d\phi$

$s = r\phi$

Sol

Since for circular arch

$$q = \frac{Vt r^2}{I} (1 - \cos\phi)$$

$$q = \frac{dF}{ds} \Rightarrow dF = q ds$$

$$\int M_0 = \int dF \cdot r = \int q ds \cdot r = \int \frac{Vt r^2}{I} (1 - \cos\phi) r d\phi$$

$$= \frac{Vt r^4}{I} \int_0^{2\pi} (1 - \cos\phi) d\phi = \frac{Vt r^4}{I} \left( \phi \Big|_0^{2\pi} - \sin\phi \Big|_0^{2\pi} \right)$$

$$= \frac{Vt r^4}{I} (2\pi - (\sin 2\pi - \sin 0)) = \frac{Vt r^4}{I} (2\pi)$$

NOW calc.  $I$

$$I_P = I_x + I_y = I_x + I_x = 2I_x$$

$$I_P = 2\pi r(t) r^2 = 2\pi r^3 t$$

$$I_x = \frac{I_P}{2} = \frac{2\pi r^3 t}{2} = \pi r^3 t$$

So

$$\frac{Vt r^4}{\pi r^3 t} (2\pi) = 2Vr = M_0$$

$\therefore M_0 = Vr$   
 $V_e = 2Vr$   
 $e = 2r$

PB 1324

REGDO

$R = 4r/\pi$

SOL

$Q = \frac{VQ}{I}$

$Q = \int xy'$

$\cos \phi = \frac{x}{r} \Rightarrow x = r \cos \phi$

$ds = r d\phi$

so  $Q = \int_0^\pi r d\phi \cdot r \cos \phi = \int_0^\pi r^2 \cos \phi d\phi$

$Q = r^2 \left[ \sin \phi \right]_0^\pi = r^2 \sin \pi$

so  $Q = \frac{V \cdot r^2 \sin \pi}{I}$

$dF = q \cdot ds$

$F = \int q \cdot ds = \int \frac{V r^2 \sin \phi}{I} \cdot r d\phi$

$= \int_0^\pi \frac{V r^3 \sin \phi}{I} d\phi = \frac{V r^3}{I} (-\cos \phi)_0^\pi$

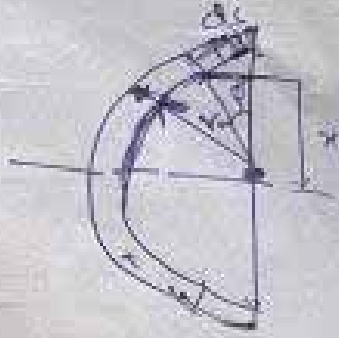
$= \frac{V r^3}{I} (-(-1 - 1)) = \frac{2 V r^3}{I}$

For Semi Circle

$I = \frac{\pi r^4}{2}$

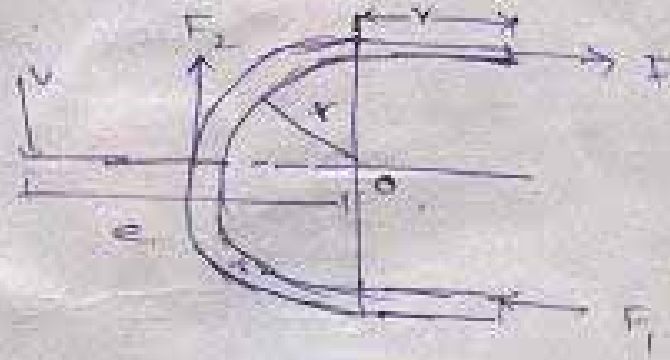
so  $F = \frac{2 V r^3}{\frac{\pi r^4}{2}} = \frac{4V}{r}$

$\sum M_0 = 0$   $V_e = \frac{4V}{\pi} r = \frac{4r}{\pi}$





Pb 13.25       $R = Q$        $E = \frac{1}{I} v^4 (x+3)$

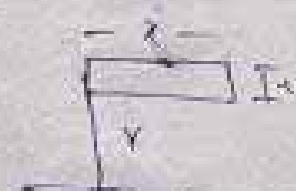


Sol

$$Q = \frac{VQ}{I}$$

~~$Q = \frac{VQ}{I} \left( \frac{2\theta}{2} \right)$~~

$Q_{1-2} = \int_0^{\theta} V \cdot x \cdot r \cdot d\theta$

$$Q_{1-2} = \frac{V \cdot x \cdot r}{I}$$


$$F = \int Q \cdot dx = \int \frac{V \cdot x \cdot r}{I} dx = \frac{V \cdot r}{I} \int_0^{\theta} x \cdot dx = \frac{V \cdot r \cdot x^2}{I \cdot 2} \Big|_0^{\theta}$$

$$\frac{V \cdot r \cdot x^2}{2I} = \frac{V \cdot r^3}{2I} = A$$

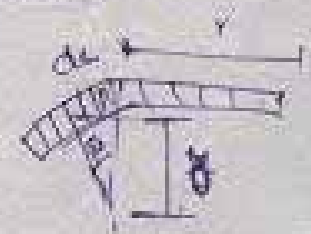
NO DO FOR  $Q_{2-3}$

$$Q_{2-3} = 0$$

$Q_{1-3} = Q_{1-2} + Q_{2-3}$

$$= V \cdot x \cdot r + d\theta \cdot r \cdot y$$

$$= r^2 + \int_0^{\theta} r \cdot \cos \phi \cdot d\phi$$

$$= r^2 + r^2 \sin \phi \Big|_0^{\theta} = r^2 + r^2 \sin \theta$$


$\cos \phi = \frac{x}{r}$

$\phi = r \cos \phi$

$d\phi = -\frac{x}{r} dx$

$$Q_{2-2} = V^3 (1 + \sin \theta)$$

So  $Q_{2-2} = \frac{V^3 (1 + \sin \theta)}{I}$

∴  $F = \int w \cdot ds$

$$= \int \frac{V^3 (1 + \sin \theta)}{I} r d\theta$$

$$= \frac{V^3 r}{I} \int_0^\pi (1 + \sin \theta) d\theta$$

$$= \frac{V^3 r}{I} \left( \theta + (-\cos \theta) \right) \Big|_0^\pi$$

$$= \frac{V^3 r}{I} (\pi - (-1 - 1))$$

$$F = \frac{V^3 r}{I} (\pi + 2)$$

Now  $M_o = 0 \rightarrow +ve$

$$V_e = 2F_1 \cdot r + F_2 \cdot r$$

$$V_e = 2 \frac{V^3 r^3}{2I} + \frac{V^3 r^3}{I} (\pi + 2) \cdot xy$$

$$= \frac{V^3 r^4}{2I} + \frac{V^3 r^4}{I} (\pi + 2)$$

$$= \frac{V^3 r^4}{I} (1 + \pi + 2)$$

$$V_e = \frac{V^3 r^4}{I} (\pi + 3)$$

$$e = \frac{V^3 r^4}{I} (\pi + 3)$$

Pb 1326 (9)

Given: 3600 lb

REQ (a) shear flow diagram  
(b) shear centre.

Sol

Sm @  $V = \frac{VQ}{I}$

$$I = \sum \left( \frac{bh^3}{12} + Ad^2 \right) = 2 \left( \frac{0.10 \times 4^3}{12} + 0.10 \times 2^2 \right) + 2 \left( \frac{4 \times 0.10^3}{12} + 0.10 \times 4 \times 2^2 \right) + \left( \frac{0.10 \times 8^3}{12} + 0.10 \times 4 \times 0 \right)$$

$$= 3.73 + 3.20 + 0.53$$

$$I = 7.46 \text{ in}^4$$

Now  $Q_{1-1}$

$$Q_{1-1} = A\bar{y} = 0.10 \times (4 - x/2)$$

$$= (0.4x - 0.05x^2) \text{ in}^2$$

So  $q_{1-1} = \frac{VQ_{1-1}}{I} = \frac{3600 \times (0.4x - 0.05x^2)}{7.46 \text{ in}^4}$

$$= 482.57 (0.4x - 0.05x^2)$$

$$q_{1-1} = 193.02x - 24.12x^2$$

(a) At  $x=0$   $q_{1-1} = 0$   
(b) At  $x=8$   $q_{1-1} = 396.64 - 156.914 = 239.726 \text{ lb/in}$

(10)

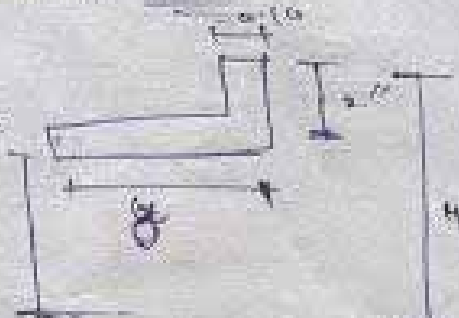
$$F = \int_0^2 v dx$$

$$= \int_0^2 193.02 x dx - \int_0^2 24.12 x^2 dx$$

$$= \frac{193.02 x^2}{2} \Big|_0^2 - \frac{24.12 x^3}{3} \Big|_0^2$$

$$= 386.04 - 64.32 = 321.72$$

Now taking section 2-2



$$Q_{2-2} = A_1 \bar{y}_1 + A_2 \bar{y}_2$$

$$= 0.10 \times 2 \times 3 + 0.10 \times 2$$

$$Q_{2-2} = 0.6 + 0.2$$

$$v_{2-2} = \frac{V Q_{2-2}}{I}$$

$$v_{2-2} = \frac{3600 (0.6 + 0.2 y)}{7.96} = 402.57 (0.6 + 0.2 y)$$

$$v_{2-2} = 209.54 + 96.514 y$$

at  $y = 0$  i.e. at  $v_{2-2} = 209.54$

at  $y = 4$  i.e.  $v_{2-2} = 209.54 + 96.514 \times 4 = 675.50$

$$F_2 = \int v dy = \int_0^4 209.54 dy + \int_0^4 96.514 y dy$$

$$= 209.54 y \Big|_0^4 + \frac{96.514 y^2}{2} \Big|_0^4$$

$$= 1150.16 + 772.08$$

$$F_2 = 1930.24$$

$$Q_{x3-3} = A_1 y_1 + A_2 y_2 + A_3 y_3$$

$$= 0.10 \times 2 \times 2 + 0.10 \times 4 \times 2 + 0.10 y \left( 2 - \frac{y}{2} \right) \left( 2 - \frac{y}{2} \right)$$

$$= 0.6 + 0.8 + 0.2y - 0.05y^2$$

$$= 1.4$$

$$Q_{x3-3} = \frac{V Q_{x3-3}}{I} = \frac{3600}{7.46} (0.6 + 0.8 + 0.2y - 0.05y^2)$$

$$= 402.57 (1.4 + 0.2y - 0.05y^2)$$

$$= 675.59 + 96.51y - 24.12y^2$$

$$a) \text{ at } y = 0$$

$$Q_{x3-3} = 675.59 \text{ lb/in}$$

$$b) \text{ at } y = 2 \quad Q_{x3-3} = 772.13 \text{ lb/in}$$

NOW FOR SHEAR CENTER  
 $M_o = 0 \rightarrow +ve$

$$V_e = 2F_1 x_1 + 2F_2 x_2$$

$$= \frac{2 \times 3214.2 \times 4 + 2 \times 1930.24 \times 2}{V}$$

$$e = 2.05 \text{ in}$$

PD 1346

GIVEN

Inside dia = 200mm

Inside radius  $= R$   
 $10.5 \text{ cm} \times 10^{-2} = 0.105 \text{ m} \approx 0.11$

$$P_1 = 60 \text{ kPa}$$
$$T_{max} = 904.0^\circ\text{C}$$

$$\underline{R_{EG}} \quad t \rightarrow b-a$$

So Finally b

$$T_{max} = \frac{b^2 P_i}{(b^2 - a^2)}$$

$$90 \times 10^6 (b^2 - 0.15^{-2}) = b^2 \times 60 \times 10^6$$

$$9ab^2 - 2.025 = 4ab^2$$

$$306^2 = 93636$$

$$p^2 = 0.0675$$

$$b = 0.25$$

$$\text{So } t = b - a = 0.25 - 0.15 = 0.10$$

⑧  $\max. S_c \text{ in hoop} = ?$

sol

Since

⑨ 
$$S_{c \max} = \frac{-2b^3 p_i}{(b^2 - a^2)}$$

$$= \frac{-2 \times (2.5)^3 (3000)}{(2.5^2 - 1.5^2)} = 9775 \text{ psi}$$

⑩ When  $p_i$  is ~~less~~ so.

$$S_c = \frac{a^2 p_i - b^2 p_o}{(b^2 - a^2)} + \frac{a^2 b^2 (p_i - p_o)}{(b^2 - a^2) r a^2}$$

$$= \frac{1.5^2 \times 10,000 - 2.5^2 \times 3000}{(2.5^2 - 1.5^2)} + \frac{(1.5^2)(2.5^2)(10,000 - 3000)}{(2.5^2 - 1.5^2) \times (1.5^2)}$$

$$= \frac{27500 - 10750}{4} + 27343.75$$

$$= 937.5 + 27343.75$$

$$= 11075 \text{ psi}$$

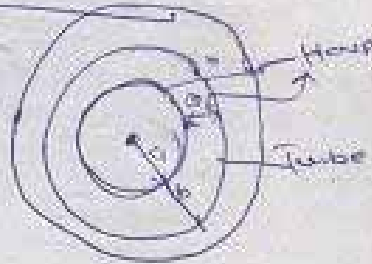
⑪ For hoop when  $p_o = 0$

$$\text{so } S_c = \frac{p_i (a^2 + b^2)}{(b^2 - a^2)} = \frac{3000 (2.5^2 + 4^2)}{(4^2 - 2.5^2)} = \frac{60750}{9.75} = 6046.15 \text{ psi}$$

Pb 1348 Given

For tube  
 $P_o = 3000 \text{ psi}$   
 $P_i = 10 \times 10^3 \text{ psi}$   
 $a = 1.5 \text{ in}$   
 $b = 2.5 \text{ in}$

For Hoop  
 $P_i = 3000 \text{ psi}$   
 $a = 2.5 \text{ in}$   
 $b = 4 \text{ in}$



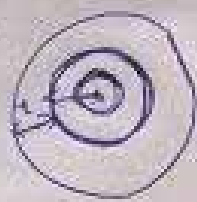
Req  $S_t = ?$  when

(A)  $S_t$  when  $P_i = 0$   
 (B)  $S_t$  when there is  $P_i$

Pb 1349

For tube  
 $t = 1''$ ,  $a = 3''$   
 $b - a = 1''$   
 $b = 4''$   
 $P_i = 4 \times 10^3 \text{ psi}$   
 Contact

For cylinder  
 $a = 2 \text{ in}$ ,  $b = 3 \text{ in}$   
 $P_o = 4 \times 10^3 \text{ psi}$



Req  $P_i = ?$

sol

$$S_t = \frac{a^2 P_i - b^2 P_o}{(b^2 - a^2)} + \frac{a^4 b^2 (P_i - P_o)}{(b^2 - a^4) a^2}$$

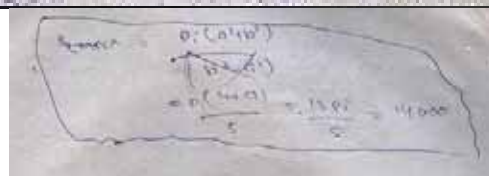
$$14 \times 10^3 = \frac{4 P_i - 9 \times 4 \times 10^3}{(9 - 4)} + \frac{9 (P_i - 4 \times 10^3)}{(9 - 4)}$$

$$P_i = 10923.07 \text{ psi}$$

$$\frac{1}{5} (13 P_i - 72000) = 14 \times 10^3$$

$$P_i = 59304.6 \text{ psi}$$

$$\frac{1}{5} (4 P_i - 36000 + 9 P_i - 36000)$$



$$P_i = \frac{14 \times 10^3 \times 5 + 36000}{13} = 10923.07 \text{ psi}$$



PD 1350      GIVEN

For shaft

$$a = 0.05 \text{ m}$$

$$b = 0.1 \text{ m}$$

$$S_{t \text{ max}} = 200 \text{ MPa} \quad (\text{contact } p)$$

Length of Hub = 200 mm = 0.2 m

$$\mu_s = 0.40$$

Req      Torque = ?

$$T = F \times L \times d$$

$$F = \mu R$$

$$R = P \times A$$

← So Finding P

For the shaft the contact p is external so for shaft

$$S_{t \text{ max}} = \frac{-2b^2 P_0}{(b^2 - a^2)} = \frac{-2(0.1^2)(P_0)}{(0.1^2 - 0.05^2)}$$

$$P_0 = \frac{200 \times 10^6 \times (0.1^2 - 0.05^2)}{-2(0.1)^2}$$

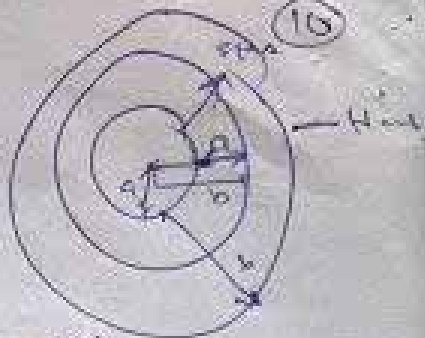
$$= 75 \text{ MPa}$$

? For Hub the contact p is internal

So max  $S_t$  is

$$S_t = \frac{P_i (b^2 + a^2)}{(b^2 - a^2)} \Rightarrow \frac{200 \times 10^6 (0.15^2 - 0.1^2)}{(0.1^2 + 0.15^2)}$$

$P_i = 76913026.11$   
 $22 \text{ MPa}$



So the sag value is 75 MPa.

So  $P = P.A$   
 $= 75 \times 10^6 \times 3 \times 1.4$   
 $= 75 \times 10^6 \times 2 \times 3.14 \times 0.1 \times 0.2$   
 $= 9420000 \text{ N}$

$F_s = \mu R = 0.4 \times 9420000$   
 $= 3768000 \text{ N}$

So  $T = F_s \times S = 3768000 \times 0.1$   
 $= \underline{\underline{376800 \text{ N.m}}}$

## SHAPE FACTOR

$$① \quad K = \frac{M_p}{M_y}$$

$$② \quad M_p = S_y \bar{z}$$

$$③ \quad M_y = S_y \bar{z}$$

$$④ \quad K = \frac{S}{I/c}$$

$$S = \frac{bh^2}{6}$$

$$I = \frac{bh^3}{12}$$

$$⑤ \quad \bar{z} = A y'$$

$$⑥ \quad S = \frac{I}{c}$$

$$I = \frac{bh^3}{12} \text{ for vertical}$$

$$\sum \left( \frac{bh^3}{12} \text{ for } + A y'^2 \right) \text{ for}$$

Horizontal member.

For diamond  $K=2$  for + rectangle 1.5

for circle = 1.7

PB

Find the shape factor & plastic moment

First of all locate N.A

$$\text{total area} = A_1 + A_2$$

$$= 14 \times 1 + 36 \times 1$$

$$A' = 50 \text{ in}^2$$

$$\text{Now } A y' = A_1 y'_1 + A_2 y'_2$$

$$A y' = 14 \times 0.5 + 36 \times 1.9 = 69.1 \text{ in}^2$$

$$y' = \frac{69.1}{50} = 1.382 \text{ in}$$

Now locate equal area axis

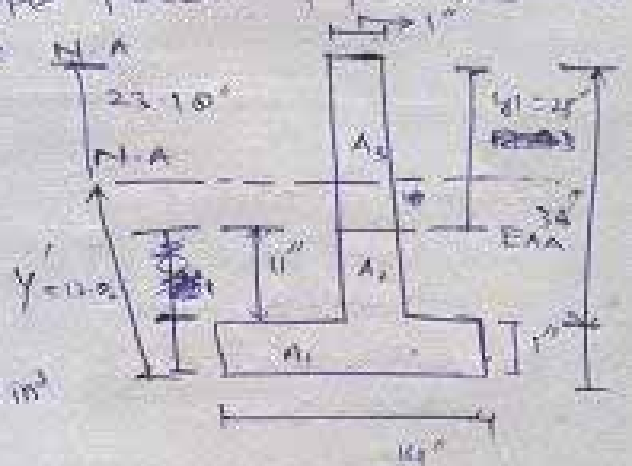
$$\begin{aligned} A_1 y'_1 &= A_2 y'_2 \\ 14 y'_1 &= 36 y'_2 \\ 14 + y'_1 &= 36 - y'_2 \\ 3 y'_2 &= 36 - 14 \\ y'_2 &= 10.67 \end{aligned}$$

$$A_2 = A_1 + A_2$$

$$y'_1 = 36 - y'_2 + 14$$

$$2 y'_1 = 50$$

$$y'_1 = 25$$



Now plastic section modulus.

②

$$Z = \sum A y_{S.A.A}$$

$$= 14 \times 1 \times 11.5 + 11 \times 1 \times 0.5 + 25 \times 1 \times 12.5$$

$$Z = 534 \text{ in}^3$$

Now moment of inertia

$$I = \frac{14 \times 1^3}{3} + \frac{1 \times 23^3}{3} + \frac{1 \times 12^3}{3} + \left( \frac{14 \times 1^3}{12} + 14 \times 1 \times (3.32)^2 \right)$$

$$= 4151.633 + 702.332 + 2405.000$$

$$= 7339.045 \text{ in}^4$$

$$S = \frac{I}{C} = \frac{7339.045}{23.10} = 316.61$$

$$K = \frac{Z}{S} = \frac{534}{316.61}$$

$$K = 1.687$$

$$M_p = S_y Z = 36 \times 534 = 19224 \text{ lb-in}$$

P10

$\Sigma A = A_1 + A_2 + A_3$

$$= 40 + 24 + 32 = 96 \text{ in}^2$$

$A \bar{y} = A_1 \bar{y}_1 + A_2 \bar{y}_2 + A_3 \bar{y}_3$

$$= 40 \times 1 + 24 \times 0 + 32 \times 10$$

$$\bar{y} = 97.41 \text{ in}$$

Now Equival area abo

$$20 \times 2 + 2(\bar{y}_1) = 2(12 - \bar{y})$$

$$20 \times 2 + A(12 - \bar{y}_1) = 40 \times 2 + 32$$

$$40 + 24 - 2\bar{y}_1 = 2\bar{y}_1 + 32 \Rightarrow \bar{y}_1 = 0$$

Now calculating plastic section

$$Z = (A y)_{E.A.A}$$

$$= 10 \times 5 + 4 \times 2 \times 2 + 8 \times 2 \times 4 + 16 \times 2 \times 9$$

$$= 560 \text{ in}^3$$

In elastic section modulus calculate

$$I = 10$$

$$I = \left( \frac{b h^3}{12} + A d^2 \right) + \frac{b h^3}{3} + \frac{b h^3}{3} + \left( \frac{b h^3}{12} + A d^2 \right)$$

$$= \left( \frac{30 \times 2^3}{12} + 30 \times 2 \times (6.41)^2 \right) + \frac{3 \times 2^3}{3} + \frac{6.50^3 \times 2}{3} + \left( \frac{16 \times 2^3}{12} + 16 \times 2 \times (7.50)^2 \right)$$

$$I = 3007.23 \text{ in}^4$$

$$S = \frac{I}{c} = \frac{3007.23}{0.50} = 6412.573 \text{ in}$$

Shape Factor  $\rightarrow k = \frac{Z}{S} = 1.201$

if  $M_{max} = S_y \times Z = 36 \times 560 = 20440 \text{ lb-ft}$

SHAPE FACTOR

- ①  $K = M_0 / M_y$     ②  $M_0 = S_y \cdot \sigma$     ③  $M_y = S_y \cdot \sigma$   
 ④  $K = \frac{S_y}{S_x} \cdot \frac{S_x}{S_y}$     ⑤  $Z = A \cdot y$     ⑥  $S = I / c$

$I = \frac{bh^3}{12}$  for vertical    &  $(\frac{bh^3}{12} + Ad^2)$  for horizontal

Pb Find S-Factor & Plastic moment

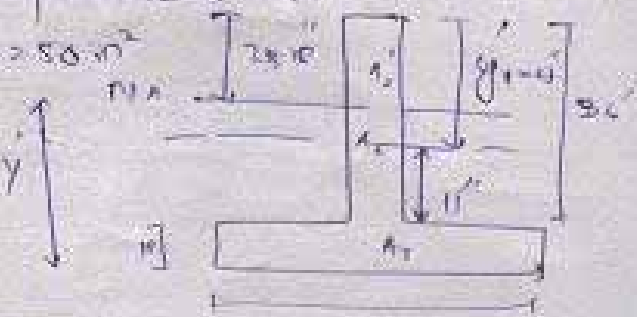
sol First of all find centroidal axis

$$A = A_1 + A_2 = 14 \times 14 + 36 \times 12 = 50 \text{ in}^2$$

$$A_y' = A_1 y_1' + A_2 y_2'$$

$$= 14 \times 10.5 + 36 \times 19$$

$$= 71$$

$$y' = 13.92 \text{ in}$$


Now local equal area axis

$$A_1 + A_2 = A_2'$$

$$14 \times 1 + (36 - y_1') = y_1'$$

$$14 + 36 = 2y_1'$$

$$y_1' = 25$$

Now finding  $Z = \frac{A_y'}{E A \cdot \sigma}$

$$Z = \frac{14 \times 1 \times 10.5 + 11 \times 1 \times 5.5 + 25 \times 1 \times 12.5}{50}$$

$$= 534 \text{ in}^3$$

Now to calculate  $S$  & find  $I_{NA}$

$$I = \left( \frac{bh^3}{12} + Ad^2 \right) + \frac{bh^3}{12} + \frac{b \cdot h_1^3}{12}$$

$$= \left( \frac{14 \times 1^3}{12} + 14 \times 1 \times 13.92^2 \right) + \left( \frac{1 \times 12^3}{12} \right) + \left( \frac{1 \times 12 \times 18^3}{12} \right)$$

$$= 2405.00 + 702.33 + 3612 + 4157.6$$

$$I = 6024.93 \text{ in}^4$$

$$\begin{aligned}
 S &= \frac{T}{C} = \frac{7339.04}{2310} = 316 \text{ C.m}^3 \\
 \text{K} &= \frac{3}{S} = \frac{3}{316} = 1.60 \\
 \text{M.P. Sg} &= 36 \times 534 = 19224 \text{ Kg}
 \end{aligned}$$

QD 2 Find S Fact MP

Sol First of all find  $A_y$

$A = 20 \times 2 + 12 \times 2 + 16 \times 2 = 96 \text{ in}^2$

$A_y = A y_1 + A_2 y_2 + A_3 y_3$

$= 20 \times 2 \times 1 + 24 \times 8 + 16 \times 2 \times 5$

$A_y = 712$

$y = \frac{712}{96} = 7.41 \text{ in}$

Now locate E A A

$20 \times 2 + (12 - y') \times 0 = 2y' + 32$

$40 + 24 - 2y' = 2y' + 32$

$y' = 4''$

Finally  $Z_x = \sum A y_1 x_1$

$Z = 20 \times 2 \times 5 + 4 \times 20 + 0 \times 2 \times 4 + 16 \times 2 \times 9$

$= 568$

$I_{NA} = \left( \frac{b_1 h_1^3}{12} + A_1 d^2 \right) + \frac{b_2 h_2^3}{3} + \frac{b_3 h_3^3}{3} + \left( \frac{b_4 h_4^3}{12} + A_4 d^2 \right)$

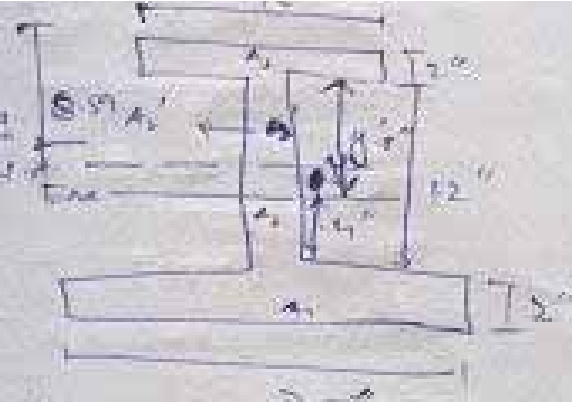
$= \left( \frac{20 \times 2^3}{12} + 20 \times (7.41)^2 \right) + \left( \frac{2 \times 591}{3} \right) + \left( \frac{2 \times 6.59^3}{3} \right) + \left( \frac{16 \times 2^3}{12} + 16 \times (7.41)^2 \right)$

$= 1656.89 + 105.84 + 190.79 + 1054.1$

$I = 3007.26 \text{ in}^4$


$S_x = \frac{I}{x} = \frac{3007.26}{0.59} = 443.22$

$\therefore \frac{Z}{S} = \frac{568}{443.22} = 1.28 \text{ in}, S_y = 2$





SHEAR FLOW & SHEAR CENTRE

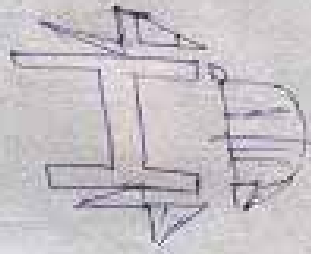


$q = \frac{VQ}{I}$

FOR WIDE FLANGE

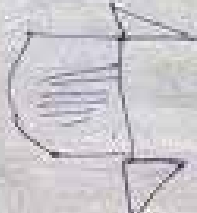
$q = \left( \frac{VQ}{I} \right) \times 2$

FOR CHANNEL SECT.



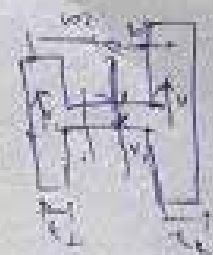
$e = \frac{h^3 b^4}{4I}$

FOR CIRCULAR



$q = \frac{VQ}{I} (1 - \cos \theta)$

FOR UNCOUPLED H-SECTION



$\frac{I_1}{h_1} = \frac{I_2}{h_2}$   
 $\frac{I_1}{h} = \frac{I_2}{h}$   
 $\frac{D_1}{b^2} = \frac{I_2}{I_1}$

$h_1 b_1 = h_2 b_2$

MASOOD AKHTAR  
Batch 3  
2007

THICK WALLED CYLINDER

$\frac{t}{r} > \frac{1}{10}$

①  $S_r = A - \frac{B}{r^2}$   
 $S_t = A + \frac{B}{r^2}$

$$A = \frac{a^2 p_i - b^2 p_o}{(b^2 - a^2)}$$

$$B = \frac{a^2 b^2 (p_i - p_o)}{(b^2 - a^2)}$$

So  $S_r = \frac{a^2 p_i - b^2 p_o}{(b^2 - a^2)} - \frac{a^2 b^2 (p_i - p_o)}{(b^2 - a^2) r^2}$

$$S_t = \frac{a^2 p_i - b^2 p_o}{(b^2 - a^2)} + \frac{a^2 b^2 (p_i - p_o)}{(b^2 - a^2) r^2}$$

CASE I INTERNAL P ONLY, i.e.  $p_o = 0$ .

So

$$S_r = \frac{a^2 p_i}{(b^2 - a^2)} \left( 1 - \frac{b^2}{r^2} \right)$$

$$S_r = \frac{a^2 P_i}{(b^2 - a^2)} \left( 1 + \frac{b^2}{r^2} \right)$$

when  $r = a$  then

$$S_r = \frac{a^2 P_i}{(b^2 - a^2)} \left( \frac{a^2 + b^2}{a^2} \right) = -P_i \text{ max}$$

∴  $S_r = \frac{a^2 P_i}{(b^2 - a^2)} \left( \frac{a^2 + b^2}{a^2} \right) = \frac{P_i (a^2 + b^2)}{(b^2 - a^2)} \text{ max}$

q. when  $r = b$

$$S_r = 0 \quad \text{∴} \quad S_r = \frac{a^2 P_i}{(b^2 - a^2)} (1 + 1) = \frac{2a^2 P_i}{(b^2 - a^2)} \text{ min}$$

Shew  $T_{\text{max}} = -\frac{b^2 P_i}{b^2 - a^2}$

CASE 2  
EXTERNAL P only  $P_i = 0$

$$S_r = -\frac{b^2 P_o}{b^2 - a^2} \left( 1 - \frac{a^2}{r^2} \right)$$

$$S_r = -\frac{b^2 P_o}{b^2 - a^2} \left( 1 + \frac{a^2}{r^2} \right)$$

when  $r = a$

$$S_r = 0 \quad \text{∴} \quad S_r = -\frac{b^2 P_o}{(b^2 - a^2)} (2) = -\frac{2b^2 P_o}{(b^2 - a^2)} \text{ max}$$

when  $r = b$

$$S_r = -\frac{b^2 P_o}{(b^2 - a^2)} \left( \frac{b^2 + a^2}{b^2} \right) = -P_o \text{ max}$$

$$S_r = -\frac{b^2 P_o}{(b^2 - a^2)} \left( \frac{b^2 + a^2}{b^2} \right) = -\frac{P_o (b^2 + a^2)}{(b^2 - a^2)} \text{ min}$$